WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY



SECTION A-11

BETHESDA STATION - SOUTH ENTRANCE CONCEPT PE SUBMITTAL CONTRACT NO. XXXX

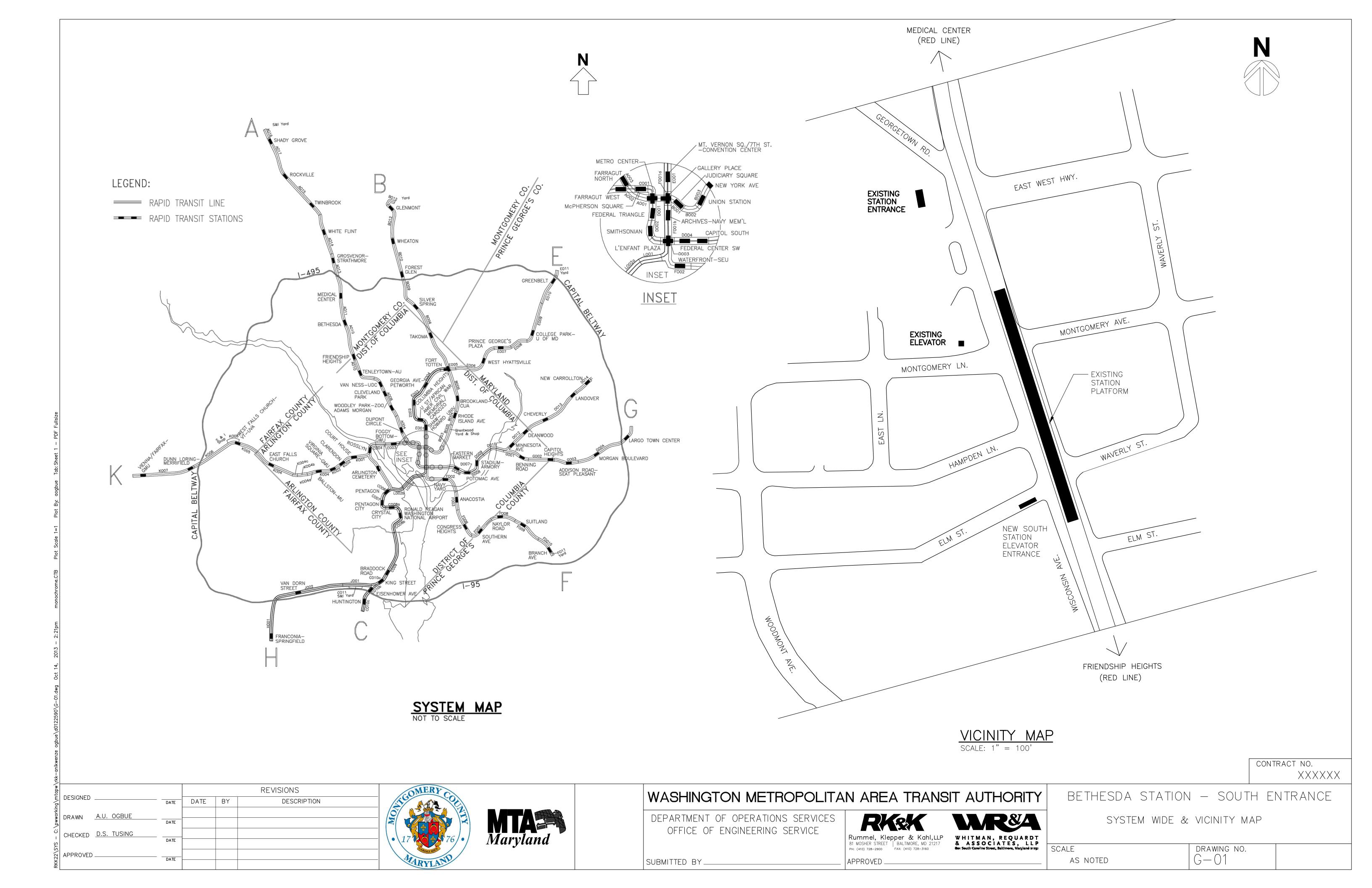
OCTOBER 2013



MONTGOMERY COUNTY
DEPARTMENT OF TRANSPORTATION
RUMMEL, KLEPPER & KAHL, LLP
IN ASSOCIATION WITH
WHITMAN, REQUARDT & ASSOCIATES, LLP







DWG. NO.	DWG. TITLE
G-00 G-01 G-02	COVER SHEET SYSTEM WIDE AND VICINITY MAP INDEX OF DRAWINGS
C-01 C-02 C-03	CIVIL NOTES, ABBREVIATIONS AND LEGEND WMATA MONUMENT RECORD SHEET ALIGNMENT SCHEMATIC & HORIZONTAL CONTROL GEOMETRY
SP-01 SP-02	EXISTING SITE PLAN ELM STREET LEVEL EXISTING SITE PLAN PURPLE LINE/TRAIL LEVEL
S0-01 S0-02 S0-03 S0-04	BORING LOG - 1 BORING LOG - 2 BORING LOG - 3 BORING LOG - 4
PP-01	VERTICAL PROFILE ELM STREET
U-01 U-02 U-03 U-04	EXISTING UTILITY COMPOSITE - 1 EXISTING UTILITY COMPOSITE - 2 PEPCO EXISTING UTILITY COMPOSITE - 3 VERIZON EXISTING UTILITY COMPOSITE - 4 WASHINGTON GAS
S-1 S-2 S-3 S-4 S-5 S-6 S-7 S-8 S-9 S-10 S-11 S-12 S-13 S-14 S-15 S-16 S-17 S-18 S-19 S-20 S-21 S-22 S-23 S-24 S-25 S-24 S-25 S-26 S-27 S-28 S-29 S-30	STRUCTURAL GENERAL NOTES STRUCTURAL ABBREVATION AND LEGEND BETHESDA STATION PLATFORM DEMOLITION PLAN AND SECTIONS STA. 391+07 TO STA. 389+07 BETHESDA STATION PLATFORM DEMOLITION SECTIONS PLATFORM DEMOLITION SECTIONS AND DETAILS PRECAST VAULT DEMOLITION DETAILS AT SOUTH PASSAGEWAY PRECAST VAULT DEMOLITION DETAILS AT SOUTH PASSAGEWAY PRECAST VAULT DEMOLITION DETAILS AT SOUTH PASSAGEWAY BETHESDA STATION PLATFORM PLAN STA. 391+07 TO STA. 389+07 BETHESDA STATION PLATFORM SECTIONS PLATFORM SECTIONS AND DETAILS PLATFORM SECTIONS AND DETAILS PLATFORM SECTIONS AND DETAILS SECTION AT SOUTH PASSAGEWAY VAULT DETAILS AT SOUTH PASSAGEWAY PRECAST VAULT AND PLATFORM DETAILS DETAILS AT SOUTH PASSAGEWAY DETAILS AT SOUTH PASSAGEWAY PROPOSED PASSAGEWAY PLAN PROPOSED PASSAGEWAY PLAN PROPOSED PASSAGEWAY SECTIONS ELEVATOR MACHINE ROOM FLOOR PLAN MEZZANINE LEVEL FLOOR PLAN WATERPROOFING SYSTEM DETAILS — 1 WATERPROOFING SYSTEM DETAILS — 2 WATERPROOFING SYSTEM DETAILS — 3 WATERPROOFING SYSTEM DETAILS — 5 WATERPROOFING SYSTEM DETAILS — 5 WATERPROOFING SYSTEM DETAILS — 5 WATERPROOFING SYSTEM DETAILS — 6 BETHESDA STATION TEMPORARY SUPPORT FOR MEZZANINE CONSTRUCTION STA. 391+07 TO STA. 389+07 TEMPORARY SUPPORT FOR MEZZANINE CONSTRUCTION STA. 391+07 TO STA. 389+07
ST-S-001 ST-S-004 ST-S-007 ST-S-009 ST-S-021 ST-S-022	CUT AND COVER SECTIONS TYPICAL DETAILS AND REINFORCEMENT DRAINAGE AND VENTILATION STRUCTURES TYPICAL DETAILS AND REINFORCEMENT ELECTRICAL BONDING OF REINFORCING STEEL, SECTIONS AND DETAILS SHEET 1 OF 2 CRITERIA FOR THE DESIGN OF TEMPORARY STRUCTURES ELECTRICAL BONDING OF REINFORCING STEEL SECTIONS AND DETAILS, SHEET 2 OF 2 TYPICAL ELECTRICAL BONDING FOR STRUCTURES

DMC	TITLE
DWG.	

DWG. NO.

CONTRACT NO.

 $\times \times \times \times \times \times$

(mtapw)					REVISIONS	
_		DATE	DATE	BY	DESCRIPTION	
pwworking	DRAWN A.U. OGBUE	DATE				
(;) (;)	CHECKED D.S. TUSING	DATE				
KKK22\SYS	APPROVED	DATE				







DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

SUBMITTED BY_





BETHESDA STATION - SOUTH ENTRANCE

INDEX OF DRAWINGS

SCALE

DRAWING NO. G-02 AS NOTED

CIVIL NOTES

- 1. FOR THE CONVENIENCE AND INFORMATION OF BIDDERS, PRINTS OF THE PLANS FOR EXISTING PERTINENT STRUCTURES ARE INCLUDED WITH THIS CONTRACT. NO RESPONSIBILITY FOR THEIR ACCURACY OR COMPLETENESS IS ASSUMED BY MONTGOMERY COUNTY. DIMENSIONS, DETAILS, ETC. AS SHOWN THEREON MAY NOT BE AS—BUILT.
- 2. BASELINE AND STATIONING FOR THE INBOUND TRACK ARE BASED ON ORIGINAL A11b CONTRACT DRAWINGS.

DATUM PLANES - WASHINGTON METROPOLITAN AREA **ELEVATION RELATIVE TO** PROJECT DATUM (FEET) DATUM WASHINGTON AQUEDUCT AND FILTRATION PLANTS (W.A.D.) NORTH AMERICAN VERTICAL DATUM (NAVD 1988) 0.717 DISTRICT OF COLUMBIA ENGINEERING DEPARTMENT Potomac Electric Power Company Washington Gas Company C. & P. Telephone Company D. C. Engineering Departments PENNSYLVANIA RAILROAD -0.00 - Project datum = sea level datum (NGVD 1929 General adjustment) U.S. Coast & Geodetic Survey U.S. Geological Survey Naval Research Laboratory (Bellevue) R. F. & P. Railroad B. & O. Railroad (Alexandria Branch) Arlington County SEA LEVEL DATUM (1912 GENERAL ADJUSTMENT) * Washington Suburban Sanitary Commission * Montgomery County LOW WATER DATUM — WASHINGTON HARBOR (L.W.D.) Baltimore District, Corps of Engineers (Except Washington Aqueduct) National Park Service Public Roads Administration Washington National Airport BOLLING AIR FORCE BASE NAVAL GUN FACTORY

EXAMPLE:

CAPITOL BENCH MARK - APEX OF BRONZE BOLT SET IN EAST WINDOW SILL OF THE SOUTH SIDE OF THE SENATE WING OF THE U.S. CAPITOL. IT WAS PLACED IN POSITION IN JUNE 1894 AND IS INSCRIBED "CAPITOL B.M."

* Note: The Washington Suburan Sanitary Commission and Montgomery County also use sea level datum (1929 general adjustment) in some areas.

DISTRICT OF COLUMBIA ENGINEERING DEPARTMENT 89.840
PENNSYLVANIA RAILROAD 89.970

PROJECT DATUM = SEA LEVEL DATUM (1929 GEN. ADJ.)

ANACOSTIA NAVAL AIR STATION

WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DIVISION OF PLANNING
DEVELOPMENT ENGINEERING &
CONSTRUCTION
OFFICE OF THE CHIEF ENGINEER FACILITIES

DATUM PLANES
WASHINGTON METROPOLITAN AREA

90.540

WMATA DESIGN CRITERIA FIGURE 11.4 (MODIFIED)

SURVEY NOTES

- 1. TOPOGRAPHIC SURVEY WAS PREPARED BY RK&K. ALL EXISTING SURFACE FEATURES SHOWN WERE SURVEYED IN AUGUST 2008.
- 2. SEE SHEET C-04 FOR WMATA METRO CONTROL MONUMENTS NOT SHOWN ON THIS SHEET (A129R, A406 & A407).
- 3. THIS DRAWING IS BASED ON WMATA GRID HORIZONTAL COORDINATES AND THE NGVD 1929 VERTICAL DATUM.
- 4. TO CONVERT NGVD 1929 ELEVATIONS TO OTHER VERTICAL DATUMS SEE DATUM PLANES WASHINGTON METROPOLITAN AREA WMATA DESIGN CRITERIA FIGURE 11.4 (MODIFIED).

ABBREVIATIONS

AHEAD BOTTOM OF CURB BEGINNING POINT BACK CATV CABLE TELEVISION CENTERLINE CONC. CONCRETE **ELEVATION** ELEV. EΡ END POINT MAC MACADAM МО MIDDLE ORDINATE NR NOT REQUIRED NU NOT USED POINT OF CURVATURE PROP PROPOSED POVC POINT ON VERTICAL CURVE PROFILE GRADE LINE PT. POINT PΤ POINT OF TANGENT RADIUS REINFORCED CONCRETE PIPE RET RETURN RIGHT OF WAY **TANGENT** TOP OF CURB

TOP OF RAIL

LEGEND

BOLLARD BORING CLEANOUT ELECTRIC METER ELECTRIC POLE ELECTRIC MANHOLE FIRE HYDRANT GAS MANHOLE GAS METER GAS VALVE HANDBOX HANDICAPPED LIGHT POLES PETROLEUM MANHOLE PETROLEUM VALVE SAN. SEWER MANHOLE SIGN STORM DRAIN MANHOLE SURVEY TRAVERSE TELEPHONE MANHOLE

TREE GENERAL

WATER METER

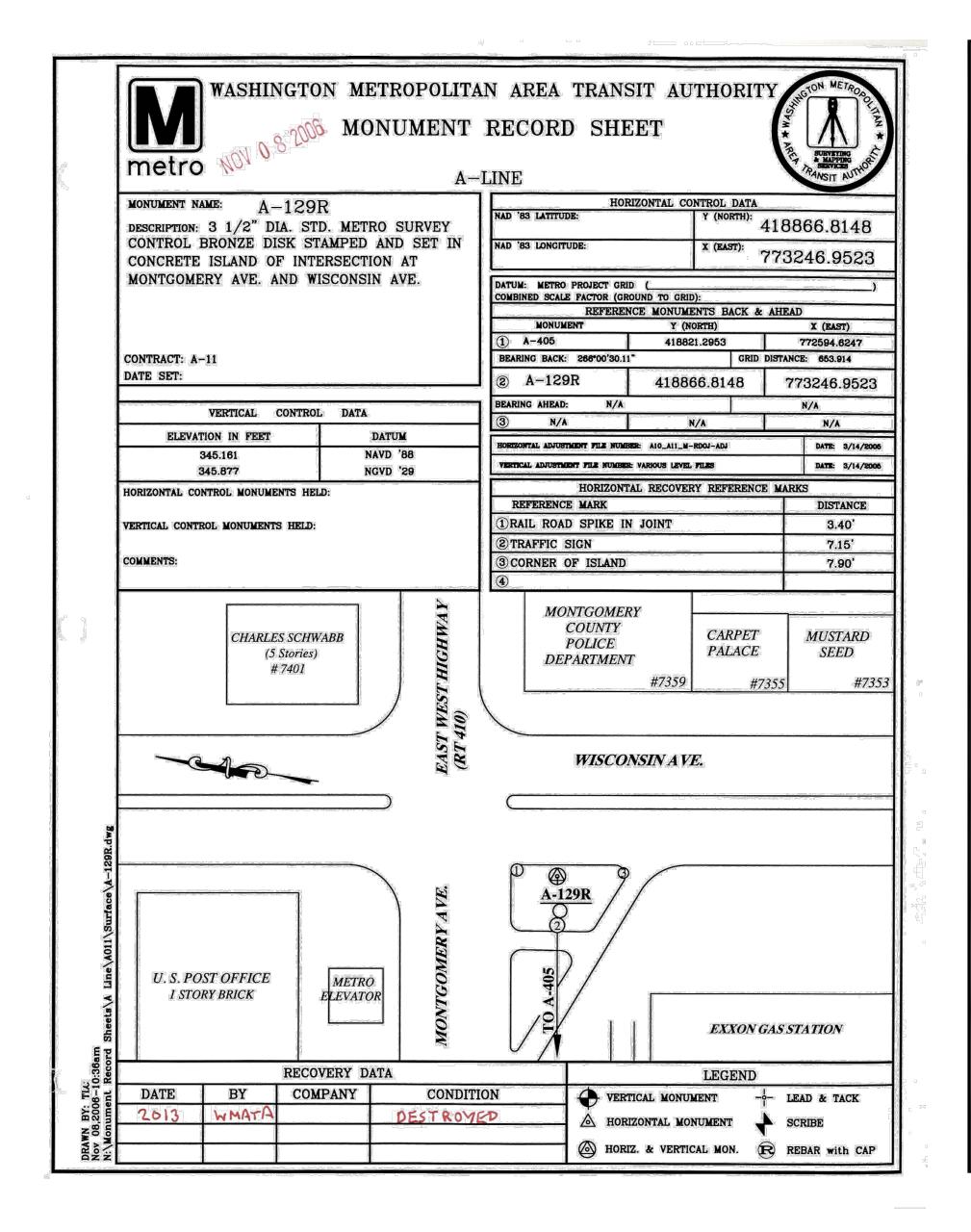
WATER VALVE

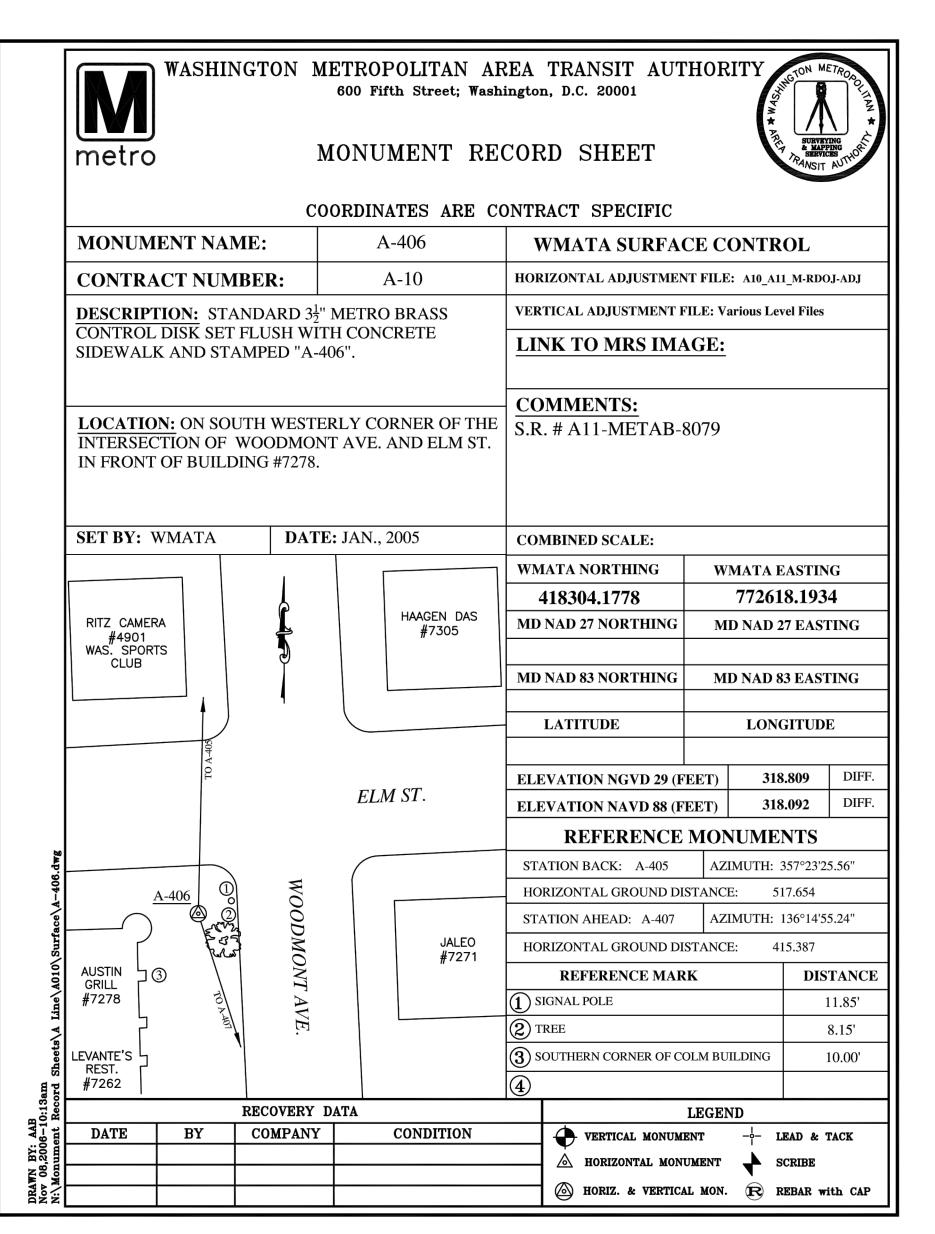
UNKNOWN MANHOLE

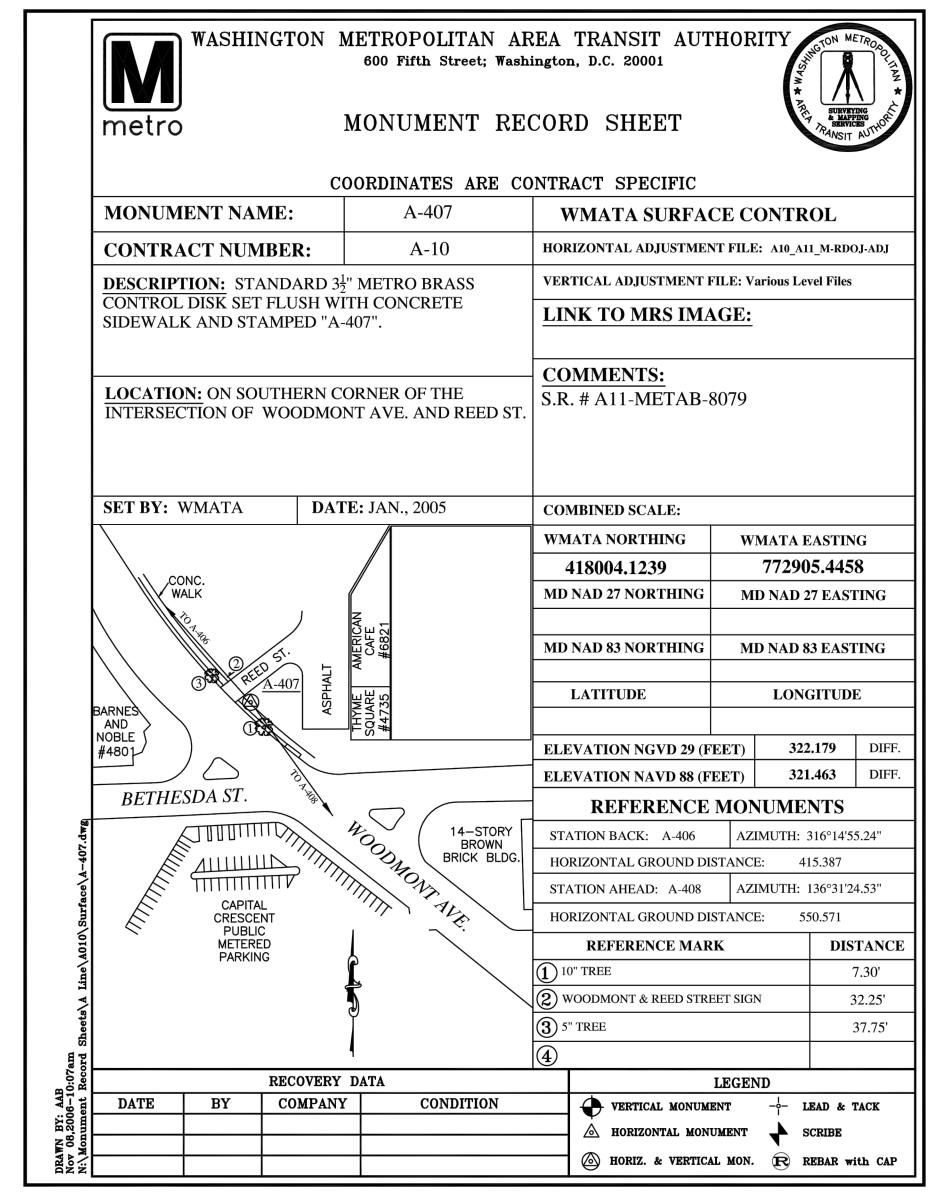
CONTRACT NO.

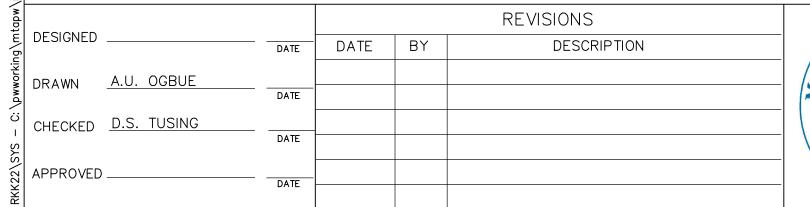
XXXXXX REVISIONS BETHESDA STATION - SOUTH ENTRANCE WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY DESIGNED _ DATE BY DESCRIPTION Maryland

Maryland DEPARTMENT OF OPERATIONS SERVICES DRAWN A.U. OGBUE RKSK CIVIL NOTES, ABBREVIATIONS AND LEGEND OFFICE OF ENGINEERING SERVICE CHECKED D.S. TUSING Rummel, Klepper & Kahl,LLP & ASSOCIATES, LLP 81 MOSHER STREET | BALTIMORE, MD 21217 SCALE DRAWING NO. PH: (410) 728-2900 FAX: (410) 728-3160 APPROVED_ C - 01AS NOTED APPROVED. SUBMITTED BY_













WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

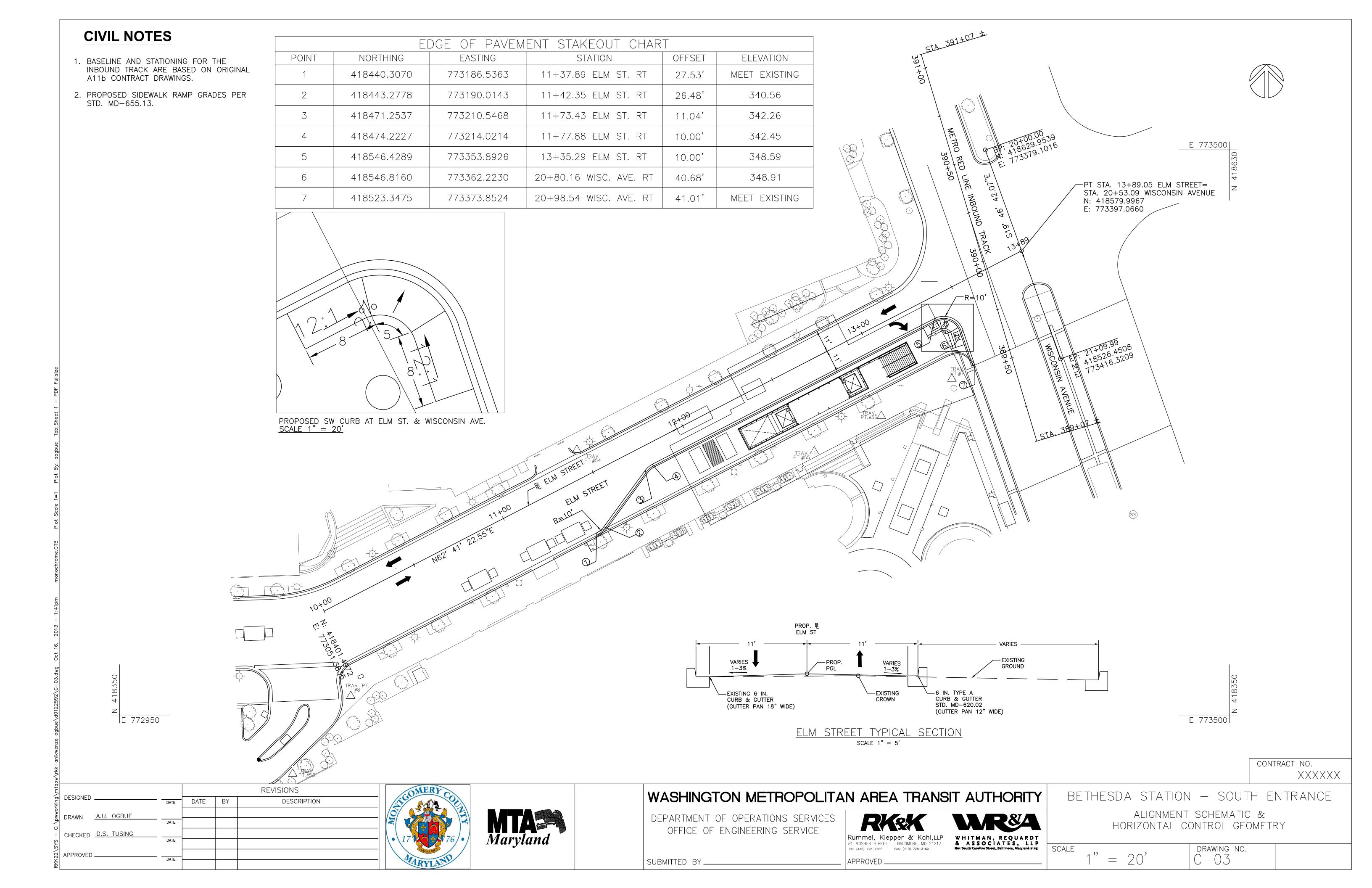
SUBMITTED BY_

Rummel, Klepper & Kahl, LLP
81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728-2900 FAX: (410) 728-3160



BETHESDA STATION — SOUTH ENTRANCE
WMATA MONUMENT RECORD SHEET

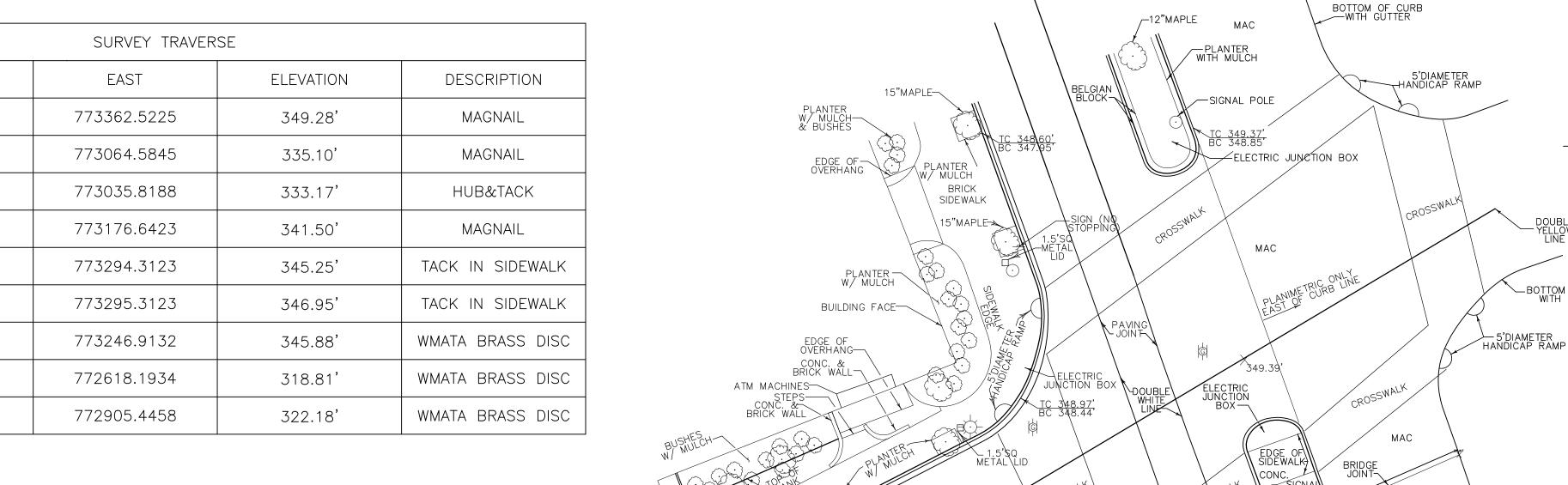
AS NOTED DRAWING NO. C-02

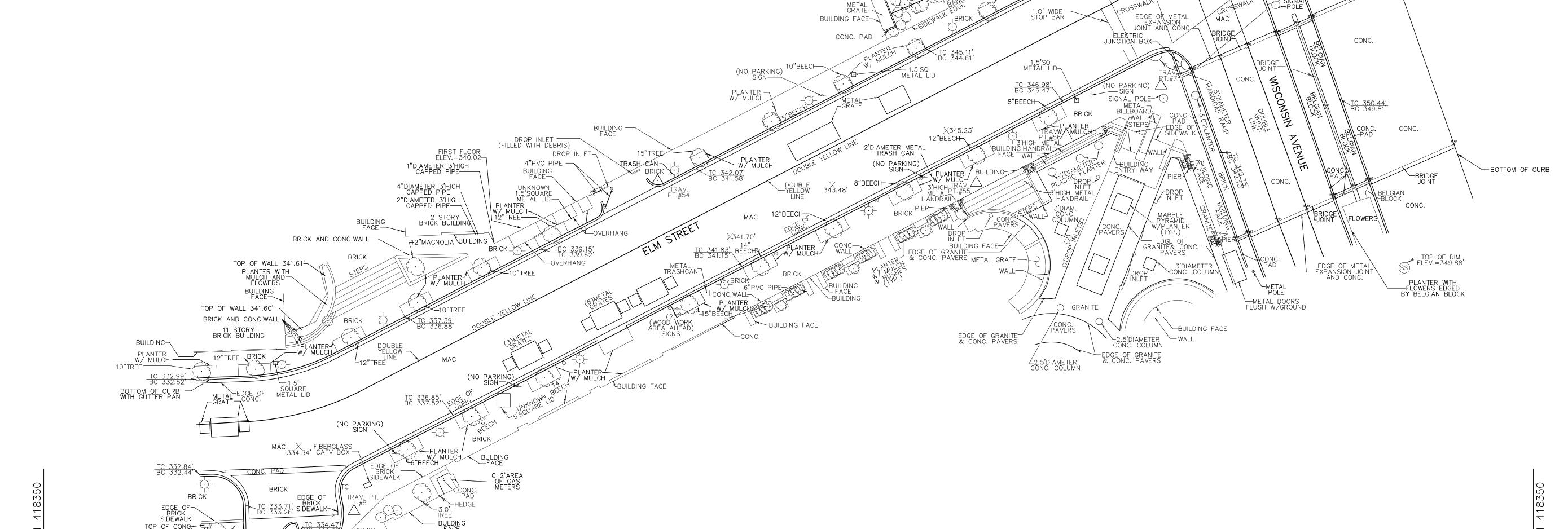


SURVEY NOTES

- 1. TOPOGRAPHIC SURVEY WAS PREPARED BY RK&K. ALL EXISTING SURFACE FEATURES SHOWN WERE SURVEYED IN AUGUST 2008.
- 2. SEE SHEET C-04 FOR WMATA METRO CONTROL MONUMENTS NOT SHOWN ON THIS SHEET (A129R, A406 & A407).
- 3. THIS DRAWING IS BASED ON WMATA GRID HORIZONTAL COORDINATES AND THE NGVD 1929 VERTICAL DATUM.
- 4. TO CONVERT NGVD 1929 ELEVATIONS TO OTHER VERTICAL DATUMS SEE DATUM PLANES - WASHINGTON METROPOLITAN AREA WMATA DESIGN CRITERIA FIGURE 11.4 (MODIFIED).

	SURVEY TRAVERSE										
TRAV. PT.	NORTH	EAST	ELEVATION	DESCRIPTION							
7	418516.5861	773362.5225	349.28'	MAGNAIL							
8	418359.4088	773064.5845	335.10'	MAGNAIL							
53	418320.8628	773035.8188	333.17'	HUB&TACK							
54	418481.3912	773176.6423	341.50'	MAGNAIL							
55	418480.6018	773294.3123	345.25'	TACK IN SIDEWALK							
56	418481.6018	773295.3123	346.95'	TACK IN SIDEWALK							
A129R	418866.7028	773246.9132	345.88'	WMATA BRASS DISC							
A406	418304.1778	772618.1934	318.81'	WMATA BRASS DISC							
A407	418004.1239	772905.4458	322.18'	WMATA BRASS DISC							





CONTRACT NO. $\times \times \times \times \times \times$

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tapw					REVISIONS	
m/g/	DESIGNED	DATE	DATE	BY	DESCRIPTION	
pwworking	DRAWN A.U. OGBUE	DATE				
SYS – C: \	CHECKED D.S. TUSING	DATE				
(22\s)	APPROVED	DATE				

1.5'SQUARE 4'HI PLASTIC BOX-BOLLARD——

E 772950



|--|

DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

SUBMITTED BY.

Rummel, Klepper & Kahl, LLP
81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728–2900 FAX: (410) 728–3160 WHITMAN, REQUARDT & ASSOCIATES, LLP APPROVED .

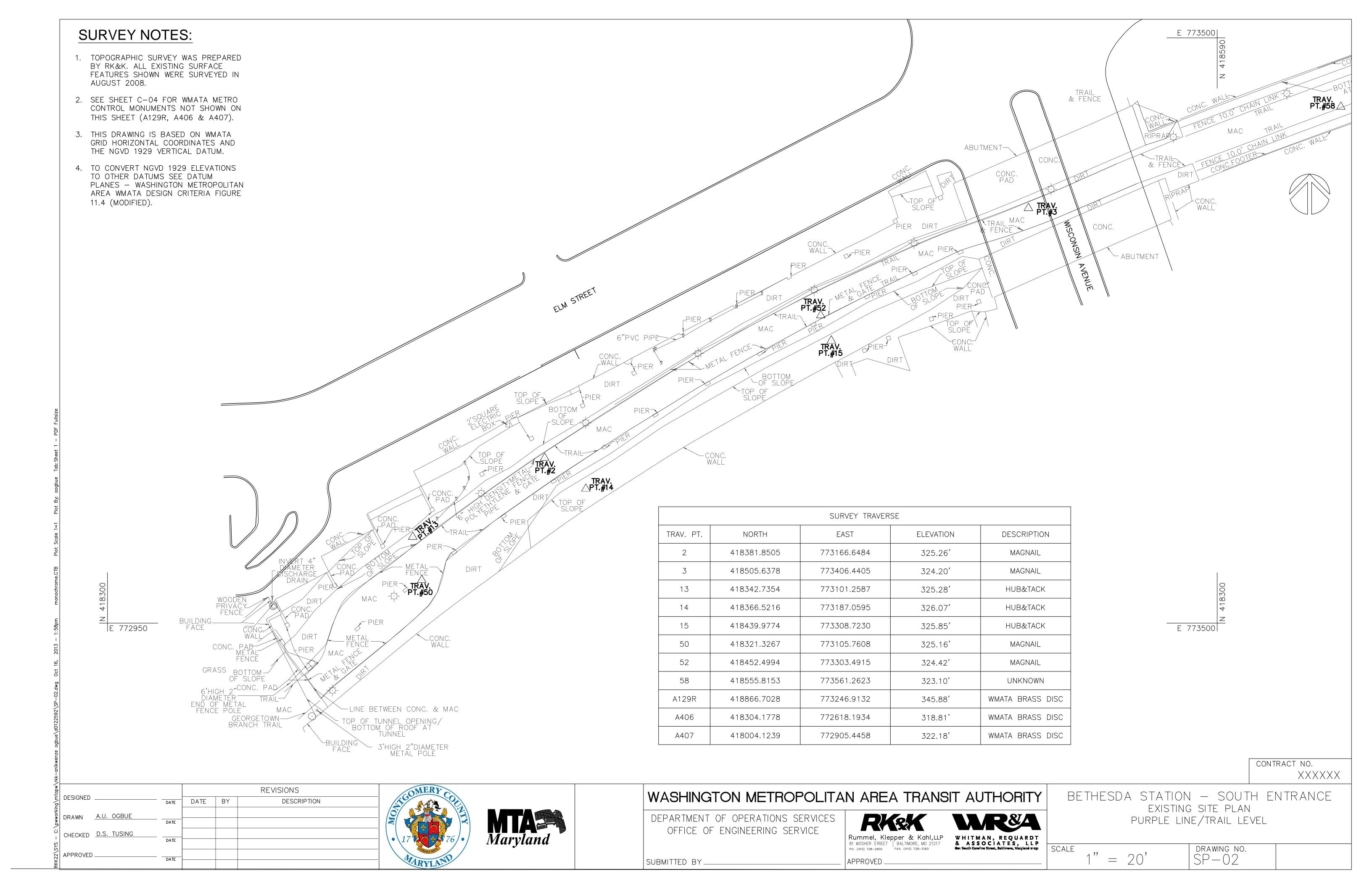
BETHESDA	STATION -	SOUTH	ENTRANCE
	EXISTING SI	TE PLAN	
	ELM STREE	T LEVEL	

E 773500

E 773500|

—BOTTOM OF CURB WITH GUTTER

1"=20'	drawing no. SP-01	

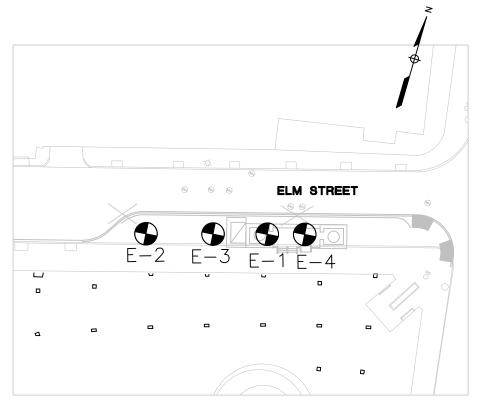


		E2CR, In	c.				BOR	.IN(GLOG
PIRCUE	ст					PROJEC			BORING NO.
SITE		South Entrance to Bethesda	Metro Statio	n	COME	PLETED	08519-04 HOLE SIZE		E - 4 GROUND ELEVATION
	Elm	Street, Bethesda	0.2	/20/09 /ATER ENC.		02/23/09 ND DRILL	AT 24 HRS		344 CAVED DEPTH
COUHL		24.59 1.1285672.51				N/A	AT 24 HHS		NIA
DRILLE	R			N/A OF HAMMER		HT OF FALL	TYPE OF CO	PE	DEPTH OF BORING
TYPE C	OF DRILL PIG & F	Ed Hill METHOD	DEPTH TO	10 lbs o rock	LOGG	00 inches SED BY:			PAGE NO.
	, ,	D-50	:	56.0		M. I	Parel		1 OF 3
рытн	STRATA STRATA	DESCRIPTION		SAMPLE	SAMPLE	EAMPLEDATA	SAMPLE LIVE AND DIAMETER	SAMPLE	REMARKS:
0	(344.0				Ф.Н	2 -	775	20 12	3" Asphalt
	343.0		-				1		9" Concrete
		Moist, Stiff, Orange Brow Silty CLAY, little Fine to Sand, little Fine to Coars	Medium 1	S-1	18"	9-7-4	DS	8.	
- 2.5 -	341.0	(Fill) Moist. Loose to Medium	Dance		_				-
		Orange Brown, Moist, fir medium SAND and SILT	ne to	S-2	18"	3+3-4	DS	10"	-
5		(SM)	u.	-					
									First 4 samples with Augers.
- 7.5 -			-	S-3	18"	3-4-5	DS	18"	Gasing was driven till 10' and
									mud sampling after that.
			-	S-4	18"	5-5-6	DS	18*	
10	333.5	Moist. Stiff to Hard, Oran	nae Brown.				-		
		Moist, mix of SILT and fi medium SAND, trace fin trace Clay, (ML/SM)	ine to	S-5	16"	7-22-50/4"	DS	16"	
-12.5-	: : :	(Saporlite)	-				-		
	- 4		-	S-6	18"	4-7-10	DS -	10"	1
- 15 -	: :		-		-				-
			-						-
100000000	1 1 1		1	S-7	18"	4-6-11	DS	8"	

CUE	E2	CR, Inc. South Entrance to Berl	BORING esida Metro Stration	LOC	PROJEC		E - 4	PAGENO. 2 OF 3
			AGIO STATION		SAMPLE DATA			± 0r 3
DEPTH	STRATA FILE/ DEPTH	DESCRIPT	SAMILLE NO.	SAMPLE	N VALLIN RQEO (%)	SAMPLE TYPE AND DIAMETER	SAMULE	RUMARKS:
0 -			S-8	18"	5-6-7	DS	10"	
.5	200		S-9	18"	7-11-15	DS	12*	
5 -	-		S-10	18"	9-14-31	DS	15"	
.5-			S-11	18"	14-22-24	DS	14*	
0 -			S-12	18°	15-20-22	DS]	16"	
2.5-	311.0	Completely Weathe Very Dense, Dark B fine to coarse SAND (SM)	rown, Grav, Drv.	17"	31-50-50/5*	DS	15"	
5								
.5-						-		
0			S-14	18"	14-20-22	DS	14	

	Е	2C	R, Inc.	BOR	ING I	LOG		BORING	NO.	E -	4
PIRCUE			,					PROJEC	T NO.	E	PAGE NO.
			South Entrance to Bethes	da Metro Statio	n				08519-04		3 OF 3
		$\overline{}$					SAMPL				
DEPTH	STRATA ELEJ DEPTH	SEADERC LOG	DESCRIPTION	s.	SAMULE NO.	SAMPLE	N VALLE	KQK) (%)	SAMPLE TYPE AND DIAMETER	SAMULE	REMARKS
42.5-	1 8		Completely Weatherer Hard, Medium to Dark	d Rock: Moist, Brown, Moist,	S-15	4"	50/	4"	- DS	4*	
45 -			SILT, Rock Fragments					-			
47.5					, S-16	1"	50/	1*	DS -	1*	Spoon Refusal @ 48.6'
52.5					S-17	1"	50/	T* /	-	, 0	Auger refusal @ 56.0'
- 55 -			End of Boring @ 56.0'	-					-		Set well @ 55'
57.5				2							
- 60 -	-								-		
62.5	-										

- BORING E-1 WAS DRILLED IN APRIL, 2008 AND BORINGS E-2 THROUGH E-4 WERE DRILLED IN FEBRUARY, 2009 BY E2CR, INC. OF BALTIMORE, MARYLAND.
- 2. N VALUE BLOWS ON A 2 INCH SPLIT BARREL SAMPLING SPOON BY A 140-LB.
 DRIVE-WEIGHT FALLING 30-INCHES INDICATING SUCCESSIVE 6 INCH INCREMENTS OF PENERTATION IN LIEU OF BLOWS PER FOOT. PENERATION LESS THAN 6 INCHES ARE INDICATED BY BLOWS OVER THE NEAREST INCH.
- 3. RQD ROCK QUALITY DESIGNATION IS DEFINED AS THE SUM OF THE LENGTH OF ROCK PIECES GREATER THAN 4 INHCES DIVIDED BY THE TOTAL CORE RUN LENGTH.
- 4. THE SOIL AND ROCK SAMPLES WERE VISUALLY CLASSIFIED IN THE FIELD AT THE TIME OF SAMPLING BY A FIELD TECHNICIAN. THE CLASSIFICATIONS WERE UPDATED BASED ON THE RESULTS OF LABORATORY TESTING AND REVIEW OF THE SAMPLES BY AN E2CR ENGINEER.



BORING LOCATION PLAN

CONTRACT NO.

DESIGNED

DESIGNED

DESIGNED

DATE

DATE

DATE

DATE

DATE

APPROVED

REVISIONS

REVISIONS

DATE

DATE

DATE

APPROVED

REVISIONS

DATE

DATE

APPROVED





DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

SUBMITTED BY

GINEERING SERVICE

Rummel, Klepper & Kahl, LI
81 MOSHER STREET | BALIMORE, MD 2121
PIN (160) 728–2000 | FAN. (190) 728–3160

APPROVED



BETHESDA STATION - SOUTH ENTRANCE
BORING LOG - 4

SCALE DRAWING NO.
AS NOTED SO-4

			E2CR, Inc.					BOR	INC	GLOG
PIFICUE	CT						PROJE	CT NO.		BORING NO.
SITE			South Entrance to Bethesda Metro	Static	n	LCCM	PLETED	08519-04 HOLE SIZE		E - 3
OHIL		Elm	F		moma		02/12/09	MOLE GIZE		
COOR	DINATES				/09/09 /ATER ENC.	AT E	ND DRILL	AT 24 HRS		342 CAVED DEPTH
DRILLE	N 47	W202.	0533 E 1485629.11	/EIGHT	N/A OF HAMMER	HEIG	N/A HT OF FALL	TYPE OF CO	DRE	N/A DEPTH OF BORING
			Ed Hill ETHOD D	1.	40 lbs o rock		30 inches SED BY:			151 PAGE NO.
TYPE 0	OF DIRILL F	iiG a M	D-50		0 ROCK 56.0	LCG		Павів		PAGE NO.
_		0	13-30		1		SAMPLEDAT			I OF /
DETH	STRATA ELE/ DBPTH	GRAPHICLOG	DESCRIPTION		SAMPLE	SAMPLE	A VALIII? RQD (%)	SAMPLE INPEAND DIAMETER	SAMPLE	REMARKS:
0	; 342.0									3" Asphalt
	341.0	****	Moist, Stiff, Tan. Clayey SILT,	little						9" Concrete
	-		fine to coarse Sand (FILL)	atue	S-1	18"	5-6-8	DS -	8.	
2.5 -										1
	-			-						1
NESCHONO.	_				S-2	18"	4-6-8	DS	113	
5										
	-							-	1	
									6"	First 4 samples with Augers.
	400				S-3	18"	4-5-6	DS	6.	Gasing was driven till 10' and
- 7.5 -	334.0			-						mud sampling
	-		Moist, Stiff, Tan, Clayey SILT, sand (ML)	some				_	-	after that.
		Ш			S-4	18"	5-5-8	DS	18*	
10	_	Ш							1	
	-	Ш						-	-	
		Ш						-		1
	_	Ш			S-5	18"	8-8-8	DS	15*	
12.5	329.0			-				+	-	1
	029.0	Ш	Moist, Very Stiff to Hard, Brow	n,				-		
			Black, Tan, SILT and Sand, tra Clay (ML)	ace .	S-6	18"	6-9-14	DS -	12"	
-	-				8-6	18"	6-9-14	DS -	12"	
- 15 -	1	Ш								1
	-							1	_	-
		Ш			S-7	18"	5-8-14	DS	12"	
_	-				1			-	1	

E2CI	R, Inc. Bo	ORING	LOG			3		
CUECT				PROJEC			PAGE NO.	
	South Entrance to Bethesda Metro	Station		SAMPLE DATA	08519-04		2 of 7	
STRATA STRATA III	DESCRIPTION	SAMPLE NU.	SAMPLE	Z VALLUG EQD (%)	SAMPLE TYPE AND DIAMINUS	SAMULE RECOVERY	RUMARKS:	
0 -		S-8	18"	6-9-11	DS	12"		
2.5		S-9	18"	7-12-21	DS	14*		
5 - 316.5		S-10	18"	6-10-10	DS	14"		
7.5-	Moist, Hard, Brown, Black, Tan, Green, Sandy SILT (ML)	S-11	18"	11-17-17	DS _	14"		
	Completely Weathered Rock: M Hard, Greenish Brown, SILT and fine to medium SAND, trace Cla trace mica (ML)	d	18"	15-34-50/4"	DS	12*		
					-			
2.5-		S-13	18"	26-50/5"	DS	11*		
305.5								
5-	Completely Weathered Rock: M Hard, Greenish Gray, Clayey Sil some Sand (ML).	oist LT,			-			
-		S-14	18"	50/2"	DS _	2"		

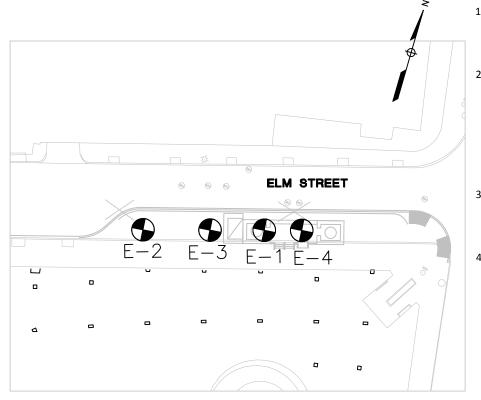
		E2C	R, Inc.	BOR	ING I	LOG			E -	3
PIRCUE	ст		South Entrance to Bethes	da Metro Statio	on.		PROJ	CT NO. 08519-04		PAGENIO. 3 OF 7
							SAMPLE DAT			
HLUMO	STRATA FILL/ DEPTH	PTII S DESCRIPTION				SAMPLE	N. VALLENY ROBO (%)	SAMPLE TYPE AND DIAMEGICK	SAMPLE RECOVERY	REMARKS:
	300.5									
42.5			Completely Weathered Hard, Greenish Brown (ML).	d Rock: Moist. , Sandy SILT				-		
- 45 -	-			-	S-15	18*	50/3"	DS	3*	
	295.5		Completely Weatherer Hard, Greenish Brown	d Rock: Moist, Clavey SII T.						
47.5		100	some Sand (ML).							
- 50 -				=	S-16	18"	50/1"	DS	1*	
52.5	-									Spoon refusal @ 53.5'
52.5					S-17	18*	50/0*	DS	0.	Auger refusal @ 55.0'
- 55 -	286.0				3-17	18	50/0	DS	,	
57.5			Black and White SCH Highly Weathered, Ve Massive, Thin and Irre 60 Degrees, Joints 45	ry Broken to quiar Foliation						All Dip Angles Measured from Horizontal.
					R-1	60*	RQD=20° 33%	RC	46%	
- 60 -	281.0		R-2: Micaceous at 65.	4 ft Inlata 75				-		
62.5	-		Degrees, 45-55 Degrees.							
JZ.J										

	E2C	R, Inc.	BOR	ING:	LOG	BORING	NO.	E - :	3		
PIRCUE						PROJEC	ET NO.		PAGE NO.		
		South Entrance to Bether	sda Metro Statio	n			08519-04		4 of 7		
	25					SAMPLE DATA	0.2	>-			
HIJE	STRATA CONTROL OF CONT	DESCRIPTIO	N	SAMULE NO.	SAMPLE	N VALLEY ROSD (%)	SAMULE TYPE AND DIAMIGINE	SAMULE RECOVERY	REMARKS:		
			-	R-2	60°	RQD=48* 80%	RC -	100%			
65	276.0		,								
		R-3: Moderately Weat 69.3-ft, Thin and Irreg 70 Degrees, Joints 45 5 Degrees.	ular Foliation						Laboratory UCC: 8,212-psi at 66.3 ft.		
-67.5		5 Degrees.		R-3	60*	RQD=52* 36%	RC -	100%	Gerchar Abrasivity Index= 3.7 at 66.3-ft. Tensile Strength- 855-psi at 66.3-ft Triaxial		
70	271.0		-						Compressive Strength= 8,233- psi at 66.3-ft		
	271.0	R-4: Moderately Weat and Highly Weathered 73-ft and from 73.4 to	from 72 to 74.1-ft, Clay				3	-			
-72.5		from 72.2 to 72.7-ft ar 74.0-ft, Thin Foliation Joints 45 Degrees, 11 Degrees, 75 Degrees 30 Degrees.	70 Degrees. 0 Degrees, 5	R-4	60*	RQD=26* 43%	RC -	100%			
75	266.0		-								
		R-5: Very Thin Foliation Joints 45-55 Degrees Degrees, 30 Degrees	10-20								
-77.5				R-5	60*	BQD=42*	BC -	100%			
80						71%	4				
90	261.0	R-6: Hard, with Soft W	Veathered								
-82.5		Zones from 82 to 82.1 82.7 to 82.8-ft, Joints Degrees, 55-60 Degr Degrees, 55 Degrees	-ft and from 30-35 ees, 75	R-6	60*	RQD=48*	RC -	100%			
				H-0	00	80%	no .	100%			
85			A 6				j.				

	E20	R, Inc.	BOR	ING	LOG	BORIN	3 NO.	E - :	3
PIRCUE		,				PROJE	CT NO.		PAGE NO.
		South Entrance to Bethesd	Metro Stati	no			08519-04	1	5 OF 7
DEPTH	STRATA OF THE COLUMN TO THE CO	DISCRIPTION		SAMPLE NO.	SAMPLE	SAMPLE DATA	SAMPLE TYPE AND DIAMEGIN	SAMPLE RECOVERY	REMARKS:
87.5	256.0	R-7: Moderately Weath Fresh, Hard. Thin Folial Begress, Joints 85 Deg Begress, 5-10 Begress Begress.	ion 60-70 rees, 45-55	R-7	60*	RQD=50° 83%	RC	100%	Tensile Strengt 1,579-psi at 88 ft Laboratory UCI 9,883-psi at 88 ft
92.5	248.0	R-8: Hard, Massive, wit Light Banding, High Mic Foltation 70-80 Degrees 80 Degrees.	a Content,	R-8	60*	RQD=47* 78%	RC -	100%	Cerchar Abrasivity Inde 4.2 at 88.5-ft. Triaxial Compressive Strength- 10.5 psi at 88.5-ft
97.5	241.0	R-9: Hard, Massive to V Irregular Foliation 80 Ck Joints 45-55 Degrees, 1 Degrees, 80 Degrees.	grees, 0-20	R-9	60*	RQD=42* 70%	RC	100%	
-102.5 - 105		R-10: hard. Very Broker Broken, with Near Vertil Irregular Follation, John Degrees, 75-85 Degree Degrees.	eal and s 45-50 s, 15-20	R-10	80°	RQD-14* 23%	RC -	- 100%	

		R, Inc.	BORING	LOG		RING NO.	E - :	3		
AUCUE	cT				PR	QUECT NO.		PAGE NO.		
		South Entrance to Bethes	sda Metro Station			08519-04		6 OF 7		
DEPJH	STRATA STRATA	DESCRIPTION	SAMILE NO.	SAMPLE	SAMPLED	SYMILE SYMILE DIAMIGUE	SAMPLE	RUMARKS		
			- R-11	1 60"	RQD=4- 73%		100%			
110	231.0	2 40 Hard Massim I	200			X.:		Laboratory UCC= 10,410-psi at 110 ft.		
112.5		R-12: Hard, Massive to Fine-Grained to Coars Joints 45-50 Degrees 20 Degrees.	se-Grained,			-		Cerchar Abrasivity Index= 3.4 at 110-ft. Tensile Strength= 800-psi at 110-ft.		
115	228.5	Gray and White. Quar Black and White, GNE Hard, Slightly Broken, Muscovite Banding, Jo	EISS, Fresh, with oints 45-50	2 60"	RQD=3' 61%	7" RC -	100%	Triaxial		
	226.0	R-13: Broken to Massi Irregular Foliation, John Degrees, 5-10 Degree	sive, with			-				
117.5		Degrees, 70 Degrees 45 Degrees.		3 60"	RQD=29 41%	5" RC -	100%			
120 -	221.0	R-14: Broken to Massi	-			-				
122.5		H-14: Broken to Mass Indistinct Foliation, Joi Degrees, 40-50 Degrees Degrees, 20 Degrees	ints 65 ees, 65			-				
			R-14	4 60"	RQD=50 83%	0" RC	100%			
- 125 -	216.0	R-15: Slightly Weather	and Marriso			-				
127.5		Goarse-Grained, Occa Banding, Foliation 60 Joints 45 Degrees.	asional Biotite			-				
			R-15	5 90"	RQD=58 96%	8" BC	100%			

PROJECT			R, Inc.	DOK	ING I	LOG	,	PPOJE	OT NO	E -	3 PAGE NO.
PHODEGI								PHUDE			
\vdash		$\overline{}$	South Entrance to Bethes	da Metro Statio	on.		C AMOUNT	E DATA	08519-04		7 OF
S 15	TRATA	GRAPHICLOG	DESCRIPTION	s	SAMPLE NO.	SAMPLE		EDATA E	SAMPLE TYPE AND DIAMEGRA	SAMULE	REMARKS:
132.5	211.0		R-16: Massive. Coars with Irregular Foliation Intrusions at 133.2-ft, Degrees, 5 Degrees.	a-Grained, , with Quartz Joints 15-20 -	R-16	90"	RQD)=52° 3%	RC -	100%	
-137.5-			R-17: Massive, Coars with Irragular Foliation Degrees, 45 Degrees Degrees.	, Joints 65	R-17	60")=46" 8%	RC -	100%	
-142.5· 145	196.0		R-18: Massive, Coars Foliation 60-70 Degre 20 Degrees, 60 Degre	es, Joints 15- 7	R-18	90°)=55°	HC -	100%	
147.5	lacc		R-19: Fresh, Broken to Coarse-Grained, Folia Degrees, Johns 15-20 Degrees, 60 Degrees.	ntion 80 Degrees, 5	R-19	9C*)=5 4 " 9%	FIG -	100%	
	7	2000	End of Boring @ 151.0	0"							
152.5									_		



- BORING E-1 WAS DRILLED IN APRIL, 2008 AND BORINGS E-2 THROUGH E-4 WERE DRILLED IN FEBRUARY, 2009 BY E2CR, INC. OF BALTIMORE, MARYLAND.
- 2. N VALUE BLOWS ON A 2 INCH SPLIT BARREL SAMPLING SPOON BY A 140-LB.

 DRIVE-WEIGHT FALLING 30-INCHES INDICATING SUCCESSIVE 6 INCH INCREMENTS OF PENERTATION IN LIEU OF BLOWS PER FOOT. PENERATION LESS THAN 6 INCHES ARE INDICATED BY BLOWS OVER THE NEAREST INCH.
- 3. RQD ROCK QUALITY DESIGNATION IS DEFINED AS THE SUM OF THE LENGTH OF ROCK PIECES GREATER THAN 4 INHCES DIVIDED BY THE TOTAL CORE RUN LENGTH.
- 4. THE SOIL AND ROCK SAMPLES WERE VISUALLY CLASSIFIED IN THE FIELD AT THE TIME OF SAMPLING BY A FIELD TECHNICIAN. THE CLASSIFICATIONS WERE UPDATED BASED ON THE RESULTS OF LABORATORY TESTING AND REVIEW OF THE SAMPLES BY AN E2CR ENGINEER.

BORING LOCATION PLAN
80ALE: T = 40"-0"

CONTRACT NO.

DESIGNED

DATE

DATE

DATE

DATE

DATE

DATE

CHECKED

DATE

APPROVED

REVISIONS

DESCRIPTION

DATE

DATE

APPROVED





DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

SUBMITTED BY



APPROVED

WHITMAN, REQUARDT ASSOCIATES, LIP

BETHESDA STATION - SOUTH ENTRANCE
BORING LOG - 3

SCALE DRAWING NO. SO-3

PRICUE	CT.		E2CR, Inc.				PROJE		INC	G LOG
rincot		,	Court Contrarens to Darkarda Ma	res Pratis			Priode			
SITE			South Entrance to Bethesda Me				PLETED	HOLE SIZE		E - 2 GROUND ELEVATION
COOR	DINATES	Elm S	treet. Bethesda	DEPTH V	V13/09 VATER END	ATE	02/17/09 ND DRILL	AT 24 HRS	:h	340.5 CAVED DEPTH
DRILLI	N 47	9185	.6667 101285597.6		N/A OF HAMME		N/A	TYPE OF CO		N/A DEPTH OF BORING
			TA DOI				HT OF FALL	TYPEOFICE	JHOE	151
TYPE	OF DRILL PR	3 a ME		DEPTH		LCG	00 inches SED BY:			PAGE NO.
_		27	D-50	53'			K. SAMPLEDAT	1 OF 7		
нда	STRATA ELE/ DEPTH	залинство	DESCRIPTION		SAMPLE	SAMPLE	N VALUITA RQD1%)	SAMPLE INPE AND DIAMETER	SAMPLE	REMARKS:
0	; 340.5									3" Asphalt 9" Concrete
- 2.5	339.5		Moist, Stiff, Light Brown, Ta Clayey SILT, trace to little fi medium Sand, presence of (FILL)	ne to	S-1	18"	3-4-4	DS	12"	a Coliciate
5			(1.122)	,	S-2	18"	5-7-7	DS	12"	
- 7.5	335.0	**	Moist, Medium Dense, Tan. Black, fine to coarse SAND. Silt, trace fine Gravel, trace (SM)	some	S-3	18"	4-6-6	DS	15"	First 4 samples with Augers. Gasing was driven till 10' and mud rotary after
10	330.0				S-4	18"	8-7-8	DS	18*	that.
			Moist, Tan, Stiff, SILT, trace presence of mica (ML)	Sand,	S-5	18"	6-7-9	DS	4*	
-12.5				-						
- 15					S-6	18"	3-6-9	DS]	12"	
13	325.0	Ш	Moist, Medium Dense to De	neo						
			Brown, White, Black, fine to SAND and SILT trace fine C trace Clay, presence of mice	coarse ravel.	S-7	18"	5-8-12	DS	12*	1

	E2C	R, Inc.	BORIN	G L	OG			E -	2	
ARCUE						PROJE			PAGE NO.	
		South Entrance to Bether	sda Metro Station			SAMPLE DATA	08519-04		2 OF 7	
DEPTH	STRATA DISTRICT CONTROL CONTRO	DESCRIPTIO	SAMILE	N	LENGTH	N VALLEY RQ0 (%)	SAMPLE TYPE AND DIAMICINE	SAMPLE	REMARKS:	
20			ş.	-8	18"	8-10-16	DS	12"		
22.5			s-	9	18"	7-15-34	DS	14"		
25	317.5	Completely Weathere Very Dense, Brown, V fine to coarse SAND of fine Gravel, trace Clay mica (SM)	Vhite, Black, and SILT trace	10	18"	18-41-50/5*	DS	15"		
			S-	11	18"	45-50/3"	DS .	9"		
27.5					02014	100400		200		
30			S-	12	18"	50/3*	DS .	3"		
32.5							-			
			8-	13	18"	50/1*	DS .	0"		
35										
37.5			- 1				-			
40			S-	14	18"	41-50/4*	DS .	8"	At 40' after drilling, the hole filled up 3-4 feel Recleaned the hole. Lost all	

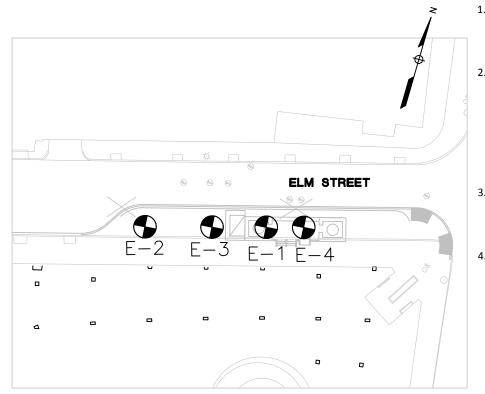
		E2CI	R, Inc.	BOR	ING I	LOG		ING NO.	E -	2
PIRCUE	ст						PRO	JECT NO.		PAGE NO.
			South Entrance to Bethesda	Metro Statio	n			08519-04		3 of 7
		LUG					SAMPLE DA		_	
DEPLIH	STRATA DID/	15	DESCRIPTION		3	걸음	20	238	38	REMARKS:
3	DEPTH	GRAPHIC		DESCRIPTION	SAMPLE NO.	SAMPLE	N VALLEY ROOT (%)	SAMPLE TYPE AND DIAMIGINE	SAMULE	
					25	N. II	z ×	& C €	× 5	0.00
	299.0									@ 40'. Drilled to 45'
	200.0	100	Completely Weathered	Rock: Moist.					1	No water
			Hard, Brown. Sandy SIL	T, trace fine					1	recovery.
42.5	1 .		to coasre Gravel (ML)	_				-	1	Drilled to 50' cleaned hole 3
	100	20							1	times then
	1 .	200								sampled.
	1 .				S-15	18*	50/11	DS	1*	No water recovery.
- 45 -	1 1	18.0		Ī						After sampling @
40 .		12								50'. dropped
		100								Auger. Refusal @ 53'
										, Johnson (M. 12
	١.	186							-	
47.5		14.		-				-		
	-	2		-					-	
		200								
	-	12.0		-	S-16	18"	50/1"	DS	12	
		134			3-16	16	50/1	US.	1	
- 50 -	1 .	Con.		-		-				
	977	13							1	
	1 .	100		-				1 '	1	
	1	15 Ac								
52.5	1 .	1							1	
52.5	287.5	4.7								
			Black and White, GNEIS							All Dip Angles Measured from
			Very Broken to Massive intrusions at 53.2-ft and	, with Quartz 54.7-ft with						Horizontal
			a Biotite Rich Band from	58.6 to	R-1	36*	RQD=20	BG BG	55%	From 53' to 115' approx 15' of
- 55 -			59.0-ft, Foliation 45-60 Joints 45-60 Degrees. 7				55%	-	1	approx 15' of water head was i
			Joints 45-60 Degrees, 7 Degrees,	u-oU.1						the hole.
	284.5		R-2: with Quartz Intrusio	no of E7 ff		-		-	-	
	-		and 58.3-ft.	nio ui Ol-II						
	1 .			-					1	
57.5				P.					1	
	1 -			-			BOD=52		1	
	1	1			R-2	60*	87%	RC	100%	
	1 -	1						1 '	1	
- 60 -										
- 6U -	1			_					1	
	279.5									
		/	R-3: Slightly Broken, Mo	derately						(100% water loss @ 56.0°)
	١.		Weathered, Slightly Stai Foliation about 60-Degre	ned.						@ 30.0)
62.5			15-20 Degrees, 45 Degr	ees, 5						
52.0	-		Degrees, 60 Degrees, 7	0-75					1	
62.5	_		15-20 Degrees, 45 Degr	ees, 5						

RATA (7.7 PPTII) PRI (South Literance to Bethess INFACENTION Degrees. Fi-4. Messalva, Unstaining 30 Cegrees, 85 Degree Degrees, 70 Degr	ed, Joints 20- es, 45-50 Stained	Three of Re-4	OO SAMPLE	SANGEDAT. 1	DESTINATION OF THE PROPERTY OF	100%	PAGENO. 4 OF 7 REMARKS: Tensile Strength 1209-psi at 88-ft Laboratory Laboratory 1209-psi at 88-ft
74.5	Uraculprion Degrees. R-4: Massive, Unstains 30 Degrees, 55 Degrees Joints from 69 to 69.5-4	ed, Joints 20- es, 45-50 Stained	TIN O	80*	RQD=62*	ATHERYS BG WARRING BG	MANULE WEGOVITRY	REMARKS: Tensile Strength 1309-psi at 68-ft
74.5	Degrees. F.4: Messive, Unstaine 30 Degrees, 85 Degree Degrees, 70 Degrees Joints from 69 to 68.5-1	ed, Joints 20- as, 45-50 Stained	R-3	80*	RQD=62*	AUDANAS BO AUDANAS BO AUDANAS BO	- 100%	Tensile Strength 1309-psi at 68-ft Laboratory by
	FI-4: Massive, Unstaine 30 Degrees, 95 Degree Degrees, 70 Degrees. Joints from 69 to 69.5-1	s, 45-50 Stained	100 27		70% RQD=52*	3	- 100%	1309-psi at 68-ft Laboratory UCC
	30 Degrees, 85 Degree Degrees, 70 Degrees, Joints from 69 to 69.5-i	s, 45-50 Stained	R-4	60°		RC -	- 100%	1309-psi at 68-ft Laboratory UCC
69.5	R.S. Marianataly Massi		ı					12,095-psi at 68- ft. Gerchar Abrasivity Index=
	to 72-ft, Massive, with Foliation, with Light Sta to 73-ft, with Occasions Grystals, Foliation 60 D	Thin Imegular aining from 72 al Large _ Degrees.		60*	RQD=55° 92%	RC	- 100%	5.3 at 68-ft. Triaxial Compressive Strength= 9,897- psi at 68-ft
59.5	Weathered, Bröken to Broken, with Biotite Bau 76,3 to 78.2-ft, with So ft and at 78.8-ft, with C Intrusions at 76.2-ft and Foliation 40-50 Degree	Slightly Inding from Inding fr	R-6	60°	RQD=46° 78%	RC .	- 100%	
-	Granitic Texture, with C Biotite Banding. Thin a Foliation 45 Degrees, Degrees, 60-70 Degre	Occasional nd Irregular Joints 45-50 es., 20-30	R-7	60"	RQD=54* 90%	RC .	- 100%	
		Joints 45 Degrees, 95 30 Degrees. 8-8: Moderately to Skig Weathered, Broken to 76, 340 78 2-8; with 78, 340 78 2-	Joints 45 Degraes, 85 Degraes, 20- 30 Degraes. Rei Moderately to Stightly Weathered. Broken to Stightly Broken, with Buttle Banding from 76.3 to 72 e-21, with 50 Fill at 78.1-81 th and at 78.8-81, with Cuartz thrustoms at 76.2 ft at 30.365 20- 30 Degraes, 45.50 Degraes, 70 Degraes.	Joints 45 Degraes, 85 Degraes, 20- 30 Degraes. R-5: Moderately to Slightly Weathered, Brither to Slightly Broken, with Blottle Bradfing from 76.5 to 78.2-t, with 50 Fill at 78.1-t fl and at 78.8-tl, with Cuartz intrusions at 78.2-t and 60.8-tl, Fellation 40-60 Degraes, Joints 20- 50 Degrees, 45-50 Degraes, 70 Degraes, 75 Degraes, 10-th 46.5-to Degraes, 80-70 Degraes, 20-30 Degraes, 80-70 Degraes, 20-30 Degraes, 80-70 Degraes, 20-30	Joints 45 Degrees, 86 Degrees, 20- 30 Degrees. R-5 90° R-5 90° R-6 Moderately to Stightly Weathered, Broken to Stightly Broken, with Elicitle Bending from 78.3 to 78.2-ft, with Soli Fill at 78.1- ft and at 78.8-ft, with Quantz Intrusions at 78.2-ft and 80.6-ft at 78.6-ft Federation 40-50 Degrees, Joints 20- 50 Degrees, 45-50 Degrees, 70 Degrees, R-7. Broken to Massive, with Grantite Texture, with Occasional Biotic Bending. This and Irragular Federation 45 Degrees, Joints 45-50 Degrees, 85-70 Degrees, 20-30	Joints 45 Degrees, 95 Degrees, 20- 30 Degrees. R-5 80° ROD=65' \$2%6' R-6' Moderately to Slightly Weathered, Broken to Slightly Broken, with District Bending from 78-3 to 78-2.H., with Soli Fill at 78-1 If and at 78-8-1, with Clearla' Rod—18-1 Federation 40-50 Degrees, Johns 20- 30 Degrees, 45-50 Degrees, 70 Degrees. R-7: Broken to Massive, with Grantine Texture, with Cocasional Grantine Texture, with Cocasional Federation 45-Degrees, 60-70 Degrees, 20-30 Degrees, 80 Degrees, 20-30	Joints 45 Degrees, 85 Degrees, 20- 30 Degrees. R-5 80° RCD_65° RC R-5 80° RCD_65° RC R-6 RCD_65° RC R-6 RCD_65° RC R-7 RCD_65° RC R-7 RCD_65° RC R-7 RCD_65° RC R-6 RCD_65° RC R-7 RCD_65° RC RCD_65° RCD_65	Joints 45 Degrees, 95 Degrees, 20- 30 Degrees. 8-5 80" BOD=55' RC 100% 8-5 92% RC 100% 8-6 Moderately to Slightly Weathward, Broken to Slightly Broken, with District Branding from 78-3 to 78-2.H., with Sol Fill at 78-1 If and at 78-8.H., with Ouantz 11 And at 78-8.H., with Ouantz 20 Degrees, 45-50 Degrees, 70 Degrees, 45-50 Degrees, 70 Degrees, 45-50 Degrees, 70 Degrees, 60-70 Degrees, 20 Degrees, 60-70 Degrees, 20 Degrees, 60-70 Degrees, 20 Degrees, 80

		E2C	R, Inc.	BOR	ING	LOG			E - :	2
PIRCUE	ст						PROJE	CT NO.		PAGE NO.
_			South Entrance to Bethes	da Metro Stati	no		SAMPLE DATA	08519-04	1	5 of 7
DEPTH	STRATA FILL/ DEPTH	GRAPHICLOG	DESCRIPTION	·	SAMPLE	SAMPLE	XAMPEDALY (%) (%)	SAMPLE TYPE AND DIAMIGTOR	SAMULE	REMARKS:
87.5	254.5		R-B: Moderately Weat Massive to Slightly Bro Granitic Texture, Thin Foliation 60-70 Degree Degrees, 70 Degrees, 60 Degrees.	oken, with and Irregular as, Joints 85	R-8	60*	RQD=42* 70%	RC	100%	Cerchar Abrasivity Index= 4.4 at 86.7-ft Tensile Strength= 1,278-psi at 86.7-ft Laboratory UCC= 10,501-psi at 86.7-ft Triaxial Compressive
92.5	244.5		R-9: Moderately West to Slightly Broken, wilf Texture, Thin and Irrej 80 Degrees, Joints 30 Degrees, 45 Degrees, 80 Degrees.	Granitic gular Foliation Degrees, 60		60*	RQD=48° 80%	RC -	100%	Strength= 8,301- psi.at 86.7-ft
97.5	239.5		R-10: Fresh, Massive, Occasional Biothe Bar 100,5 to 101-ft, Foliatie Joints 45 Degrees.	iding from on Indistinct,	R-10	60°	RQD=60° 100%	RC .	100%	
102.5	-		R-11: Massive, with C. Biotile Banding at 101- Granitic Texture, with Large Crystals, with a Inclusion at 103.3-ft, Occasional Muscovite Foliation 60 Degrees, Degrees, 70 Degrees Degrees.	ft, with Occasional Quartz _ vith Banding. Joints 20-30	R-11	80°	RQD-57* 95%	RC -	- 100%	
107.5	234.5		B-12: Fresh, Massive, Crystals, Foliation 70 I 109 to 110.5-ft, Indisti Elsewhere, Joints 20 I Degrees.	Degrees from not				-	-	Laboratory UCC= 10,706-psi at 106 ft. Tensile Strength= 1545-psi at 106-f

	E2	CR, Inc.	BOR	ING I	LOG	Bo	ORING NO.	E -	,
PIRCUE		,	2.010				POJECT NO:	E-	PAGE NO.
		South Entrance to Bethes	da Metro Statio	n			08519-04	1	6 OF 7
	- 5					SAMPLE	DATA		
DEPTH	STRATA FILL/ DEPTH	DESCRIPTION	N.	SAMILE NO.	SAMPLE	N VALLEY RQO (%)		SAMPLE	REMARKS:
110			,	R-12	60"	RQD=6 100%	80" RC	100%	Abrasivity Index= 5.1 at 106-ft. Gerchar Abrasivity Index= 5.1 at 106-ft.
	229.5	R-13: Fresh to Modera Weathered, Massive, Banding from 112 to 1	with Biotite					-	
112.5		Foliation 60 Degrees, Degrees, 60 Degrees	Joints 45	R-13	60"	RQD=5 95%		100%	
115	224.5	R-14: Moderately Wea	athered to				8		Water level at
117.5		Fresh, Massive, Fine of Foliation 60-65 Degree Irregular Elsewhere, Ji Degrees.	Grained, es at 119.3-ft.	R-14	60"	RQD=5 94%		100%	115' while coring from 116' to 150
120 -	219.5	R-15: Fresh. Massive	with				-		
122.5		Occasional Biotite Bar Indistinct Foliation, Joi Degrees, 45 Degrees.	nds, with ints 30						
				R-15	60"	RQD=6 100%		100%	
- 125 -	214.5						-		
127.5		R-16: Massive. with G Texture, with Biotite B 130.5 to 131-ft, with In Foliation.	anding from			RQD=6	54"	-	
- 130 -			1	R-16	90"	90%		100%	

PIRCUE			R, Inc.	ING			PROJECT NO.			PAGE NO.	
PINCOE	101										
_	Т		South Entrance to Bethes	ida Metro Stati	n		SAMPLE		08519-04		7 OF
DEPTH	STRATA ELEJ DEPTH	SKAPHCLOG	DESCRIPTION	×.	SAMPLE NO.	SAMPLE	SAMPLE SERVE Z		SAMPLE TYPE AND DIAMIGING	SAMULE	REMARKS:
	209.5	1005X					V -				
132.5			R-17: Massive. Fine-C Coarse-Grained, with Texture from 131 to 11 Rich from 131.3 to 13 Foliation, Joints 70-80	Granitic 31.3-ft, Mica- 2.6-ft, No	R-17	90°	RQD= 90%		RC -	100%	
<u> </u>	204.5		R-18: Fresh, Massive.								
137.5	199.5		Grained, Joints 30 De	grees.	R-18	60"	RQD= 1009		RC -	100%	
-142.5 145	-		R-19: Slightly Weather Coarse-Grained, with Follation, Joints 45 De Degrees, 5 Degrees, 1	grees, 20	R-19	60°	RQD= 84%	.50°	HG -	100%	
147.5	194.5		R-20: Fresh, Massive, Grained, with Bottle B 149 to 149-51. With In Follation, Joints 20 De Degrees.	landing from '	F-20	80"	RQD= 100°		RC -	100%	



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BORING LOCATION PLAN
SCALE: 1' • 40'-0'

CONTRACT NO.

DESIGNED DATE DATE BY DESCRIPTION

DRAWN DATE

CHECKED

APPROVED

REVISIONS

REVISIONS

DATE

DATE

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REVISIONS

DATE



MTA Maryland

DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

OFFICE OF ENGINEERING SERVICE

Rummel, Klepper & Ka
81 MOSHER STREET | BALTIMORE, MI
PIE (410) 728–2900 | FAX: (410) 728–311

SUBMITTED BY

APPROVED



BETHESDA STATION - SOUTH ENTRANCE BORING LOG - 2

SCALE DRAWING NO. SO-2

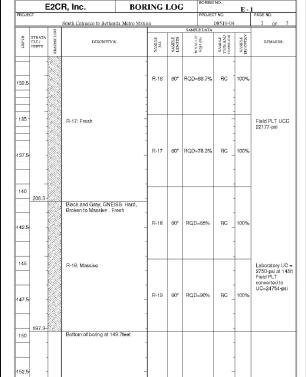
		E2CR, Inc.						BOR	INC	GLOG
PIFICUE	СТ					PF	ROUEG	TNO.		BORING NO.
		South Entrance to Bethesda Me	stro Static	n			08519-04 0 HOLE SIZE			E - 1
SITE						PLETED				GPOUND ELEVATIO
COORE	DINATES F.III	Street, Bethesda	DEPTH V	22/08 VATER ENC	. AT EN	4/24/08 ID DRILL		AT 24 HRS		347 CAVED DEPTH
DFILLE	0		HATLOUT	N/A OF HAMME	D LEGO	N/A HT OF FA		TYPE OF CO	roe:	N/A DEPTH OF BORING
		Tohnny Seic						THEORGO	THE.	149.7
TYPE C	F DRILL PIG & I		DEPTHT		LOGG	0 inche ED BY:				PAGE NO.
_	I	D-50		56.5		SAMPLE	BE	is		1 OF
ретн	STRATA STRATA	DESCRIPTION		ON	SAMPLE	A VALUE		SAMPLE INPEAND DIAMETER	SAMPLE	REMARKS:
0	347.0				7.5			7.0	ρź	5" Asphalt
	346.D									
	- 100	Dry, Very Stiff, Brown, Silty Trace Sand (CL-ML) FILL	CLAY,				-			-
	1 8			S-1	18"	6-8-	10	DS	13"	
2.5	183	3	-					_		
_	- 8	S-2: Stiff (FILL)								-
reservo				S-2	18"	4-6-	8	DS	147	
5	188							_		
-	- 83	8						-		
		S-3: Moist, Stiff, Brown, Ta	n, Little	WHY THE WAY						First 4 samples
	189	Sand (FILL)		S-3	18"	4-7-	-7	DS	13"	with Augers. Casing was
7.5 -	188									driven till 10' ar mud sampling
		3						2		after that.
	183	S-4: Moist, Very Stiff, Black Little Sand (FILL)	Brown,	1						1
-	- 488	3		S-4	18"	5-8-	10	DS	16"	
10										
	188	á								
-								-		
12.5	188	8						-		
12.0			-							
\vdash	333.5	Moist, Very Stiff, Black, Gre	en. Tan.	-						-
		SILT, Trace Mica, Trace Sa	and (ML)	S-5	18"	4-6-	10	DS	18*	
- 15 -	1 71		_							-
								0		
	1 11			1				-		
17.5	1 1111			1						

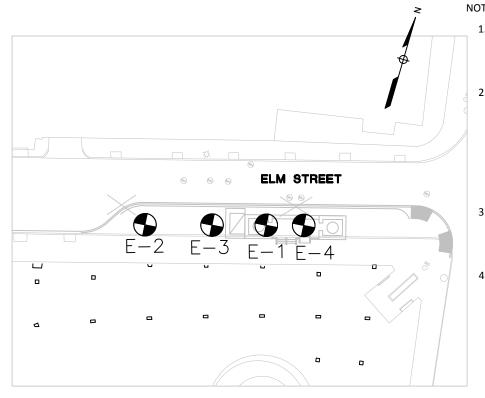
E20	CR, Inc.	BORING	LOC	PROJE		E -	PAGE NO.
	South Entrance to Bether	sda Metro Station			08519-04		2 OF 7
STRATA 5			1	SAMPLE DATA		>-	
STRATA DE	DISCRIPTION	SAMILE NO.	SAMPLE	N VALLED ROOT (%)	SAMPLE TYPE AND DIAMIGUE	SAMULE	REMARKS:
	S-6: Moist, Hard (ML)	S-6	18"	11-16-24	DS	18*	
5					×		
	8-7: Moist, Hard (ML)	S-7	18"	6-18-21	DS	18*	
5		-			-		
318.5	Gompletely Weathere	d Rock ard, Black S.8	18"	12-19-32	DS	18"	
	Green, Ian, SILI, Ira	ice Mica, is (ML)	18	12-19-32	DS .	18.	
	5	-					
5-					-		
		S-9	18"	29-20-37	DS	13*	
		-			-		
5-		-			-		
308.5	Dry, Hard, Green. Bro SILT, Trace Mica, Tra		3"	50/3"	DS	3"	
	Fragments (ML)	-1			,		

	E	E2C	R, Inc.	BOR	ING I	LOC	BOF	IING NO.	E - :	1
PIRCUE	:CT						PRO	JECT NO.		PAGE NO.
			South Entrance to Bethes-	da Metro Stati	n			08519-04		3 of 7
DEPTH	STRATA FILE/ DEPTH	GRAPHIC LOG	DESCRIPTION		SAMILE NO.	SAMPLE	SAMPLE DA	SAMILE TYPE AND	SAMILE	REMARKS:
42.5		2.00 m 5.70 m 5.80 m		-	, S-11	2"	50/2*	DS	2*	
45								-		
47.5					, S-12	2"	50/2*	DS	2*	
52.5	-			- - - - - -						
55	290.5		S-14: Wet, Hard (ML)	-	, S-13	2.5"	50/2.5*	DS	2.5*	
	200.0		Black, Brown, Green, a	and Gray,	S-14	2"	50/2*	DS		Water on Rods a
57.5			Homblende GNEISS, I Weathered, Soft to Ha	righly rd, Massive	R-1	42*	RQD=36.4		42%	56-ft Auger Refusal at 56,5-ft Coring time 10 min.
60										Coring time 10 min.
62.5		88			R-2	60°	RQD=10	% RC	28%	
	1 -	42X3							1	

		E2C	R, Inc.	BOR	ING I	LOG			E -	1
PIRCUE	CT		South Entrance to Bethesd	a Metro Static	10		PROJEC	T NO. 08519-04		PAGE NO. 4 OF 7
		9	Commented to Deliced	a sectio statit			SAMPLE DATA	00/12-04		4 UF /
DEPTH	STRATA FILE/ DEPTH	GRAPHIC LOG	DESCRIPTION		SAMULE NO.	SAMPLE	N VALLES	SAMPLE TYPE AND	SAMUE RECOVERY	REMARKS:
65	281.8		Black and Gray, Horble GNEISS, Slightly Weath Slightly Broken	nde . Jered, Hard, .	R-3	60°	RQD=73.3%	RC -	100%	Coring time 12 min.
70 -72.5					R-4	60°	ROD=56.7%	RC -	100%	Field PLT converted to UCC=12982-psi Laboratory UCC 1200-psi at 68.5 f
75 -77.5					R-5	60*	ROD=71.7%	RC -	100%	
80			R-6: Massive		R-6	60*	HQD=86.7%	RC T	100%	Field PLT converted to UCC=22498-psi
85	-		R-7: Slightly Broken to I	- Massive						

	E2C	R, Inc.	BORING	LOC	3	BORING NO.		E - :	ı
PIRCUE	ст	South Enterprise to Destrict	da Motos Station			PROJECT NO	λ 19-04		PAGE NO.
	9	South Entrance to Bethes	nonstation		SAMPLE		15-04		6 OF 7
ныя	STRATA 1000 CONTROL OF	DESCRIPTION	armers are	SAMPLE	A VALLEY Z		TYPEAND	SAMPLE RECOVERY	RUMARKS:
110			R-1	2 60*	RQD=	-75% I	ac -	100%	Field PLT UCC 34197-psi Laboratory UCC= 11780-psi at 109- ft Laboratory UCC- 11330-psi at 109.1-ft
120		R-13: Massive to Brok	en R-1	3 60"	RQD=	75%	RC]	100%	Field PLT UCC 27845-psi Leboratory UCC= 13390-psi at 119- ft.
122.5			R-1	4 60"	RQD=8	33.3%	ac]	100%	
127.5			R-1	5 54"	RQD=9	98.7% I	RC .	100%	Field PLT UCC 31510-psi Laboratory UCC=





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BORING LOCATION PLAN 8CALE: 1" - 40"-0"

CONTRACT NO. XXXXXX

REVISIONS DESIGNED DATE BY DESCRIPTION DRAWN CHECKED APPROVED





DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

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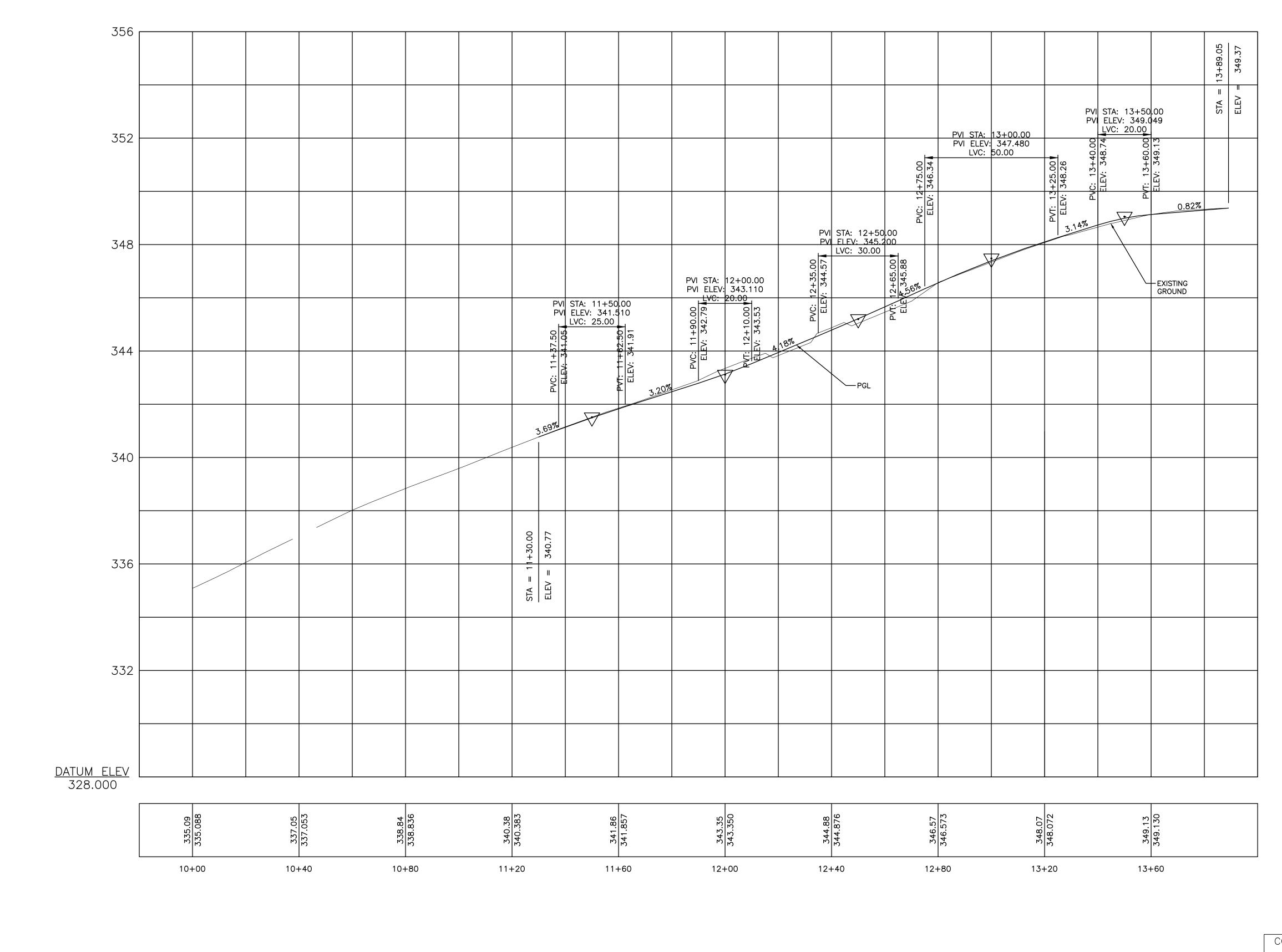


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BETHESDA STATION - SOUTH ENTRANCE BORING LOG - 1

SCALE DRAWING NO. SO - 1AS NOTED



CONTRACT NO. $\times \times \times \times \times \times$

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DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

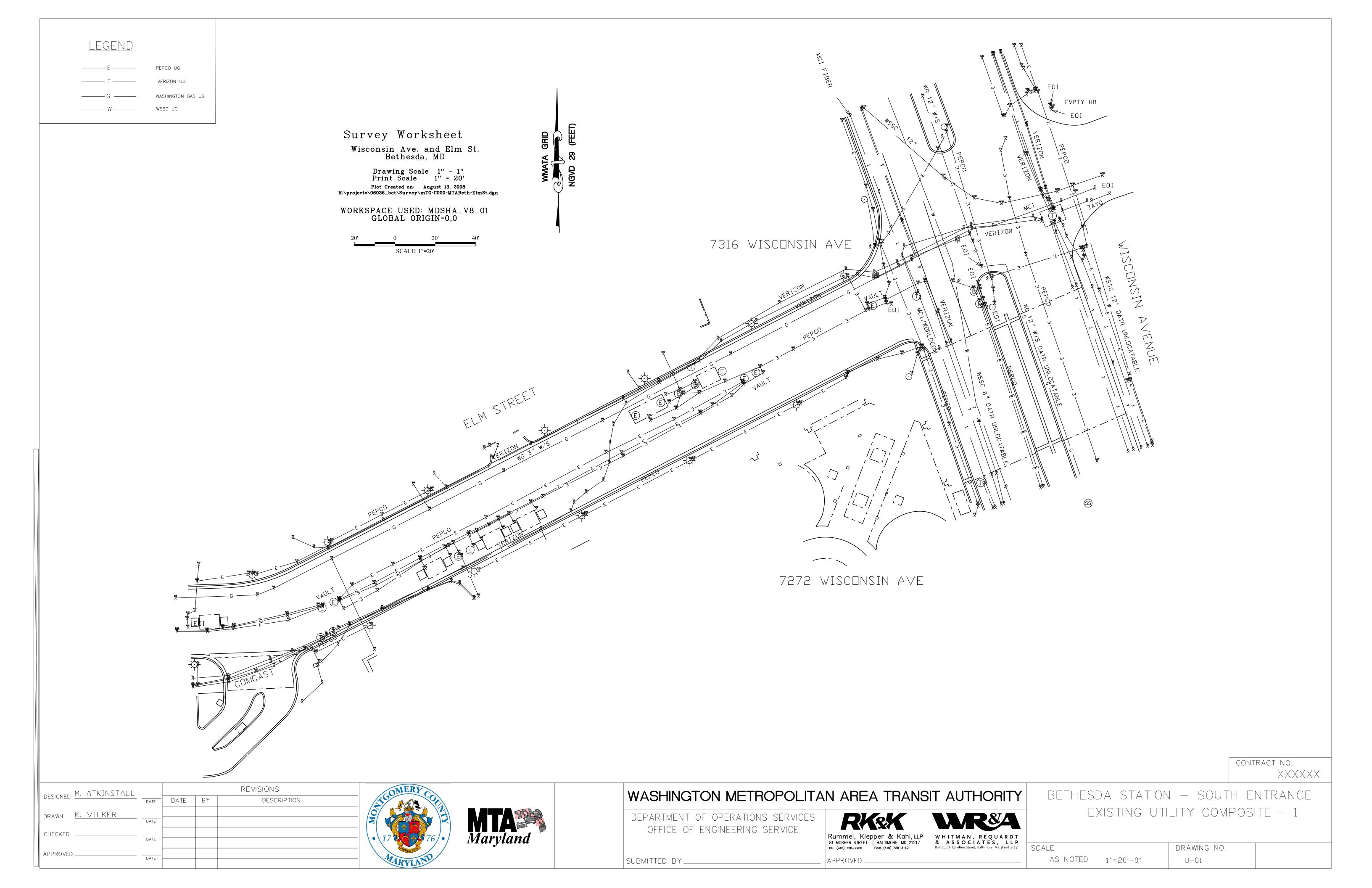
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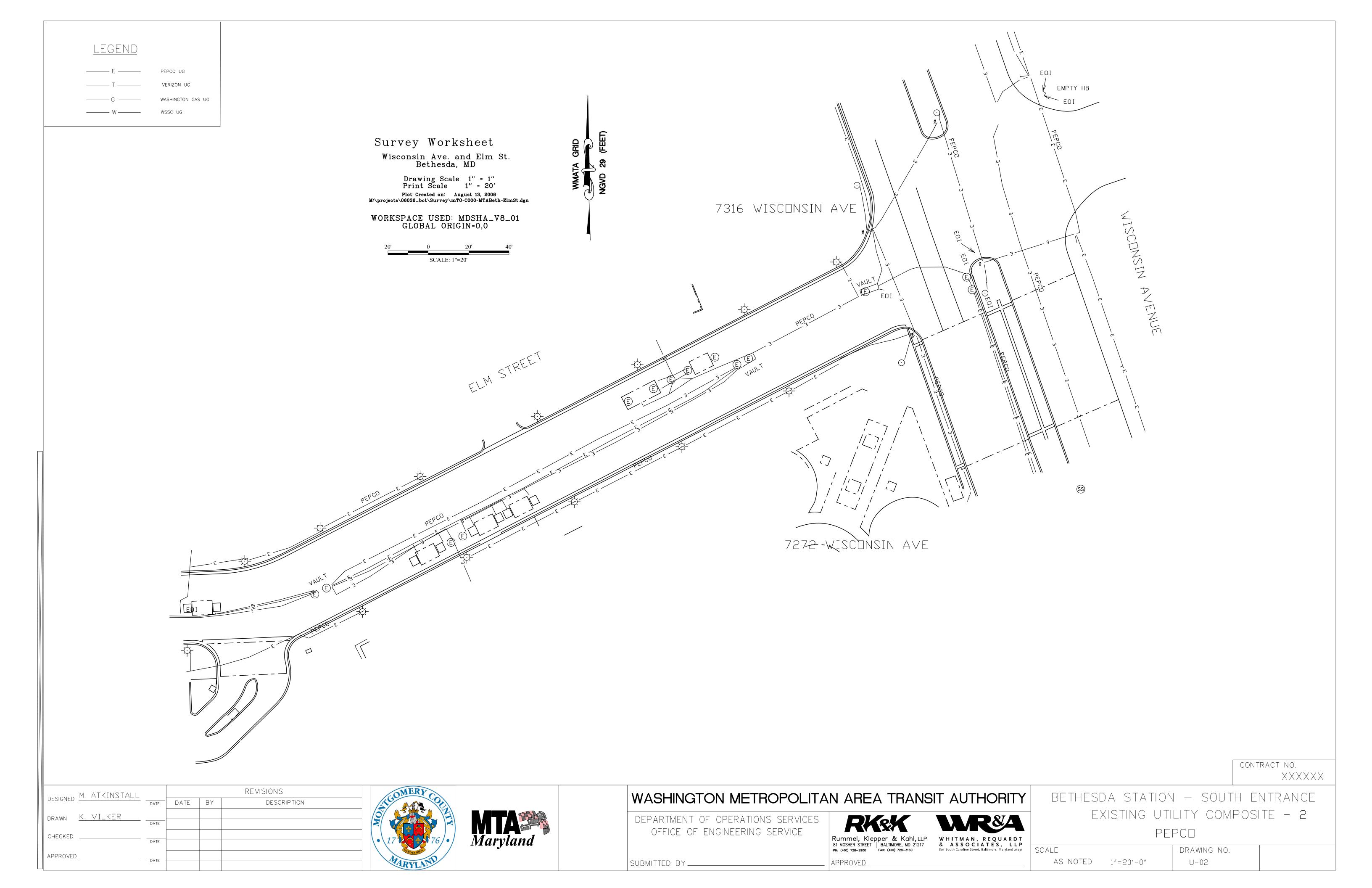
Rummel, Klepper & Kahl, LLP
81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728-2900 FAX: (410) 728-3160 WHITMAN, REQUARDT
& ASSOCIATES, LLP
301 South Caroline Street, Baltimore, Maryland 21231

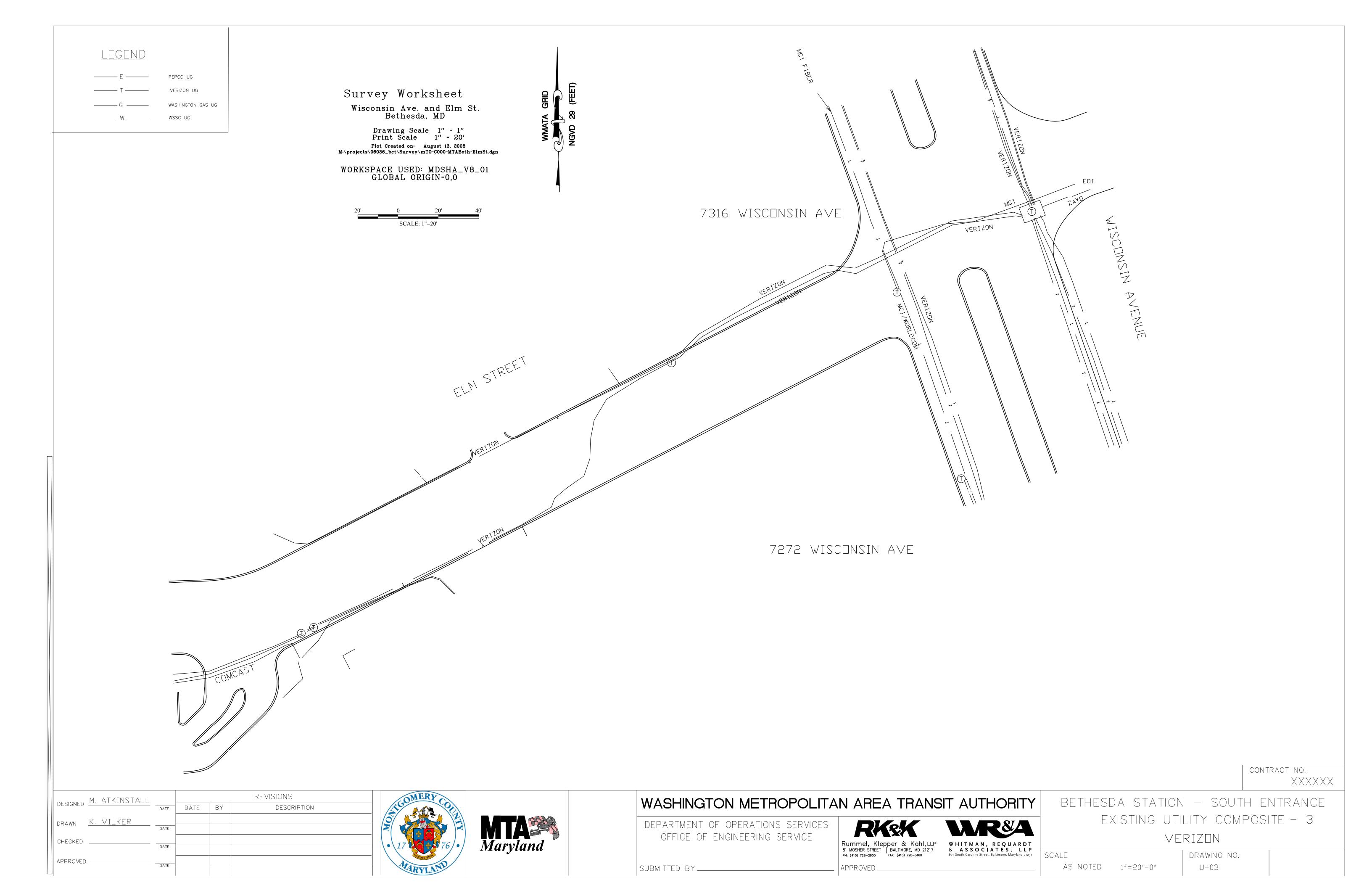


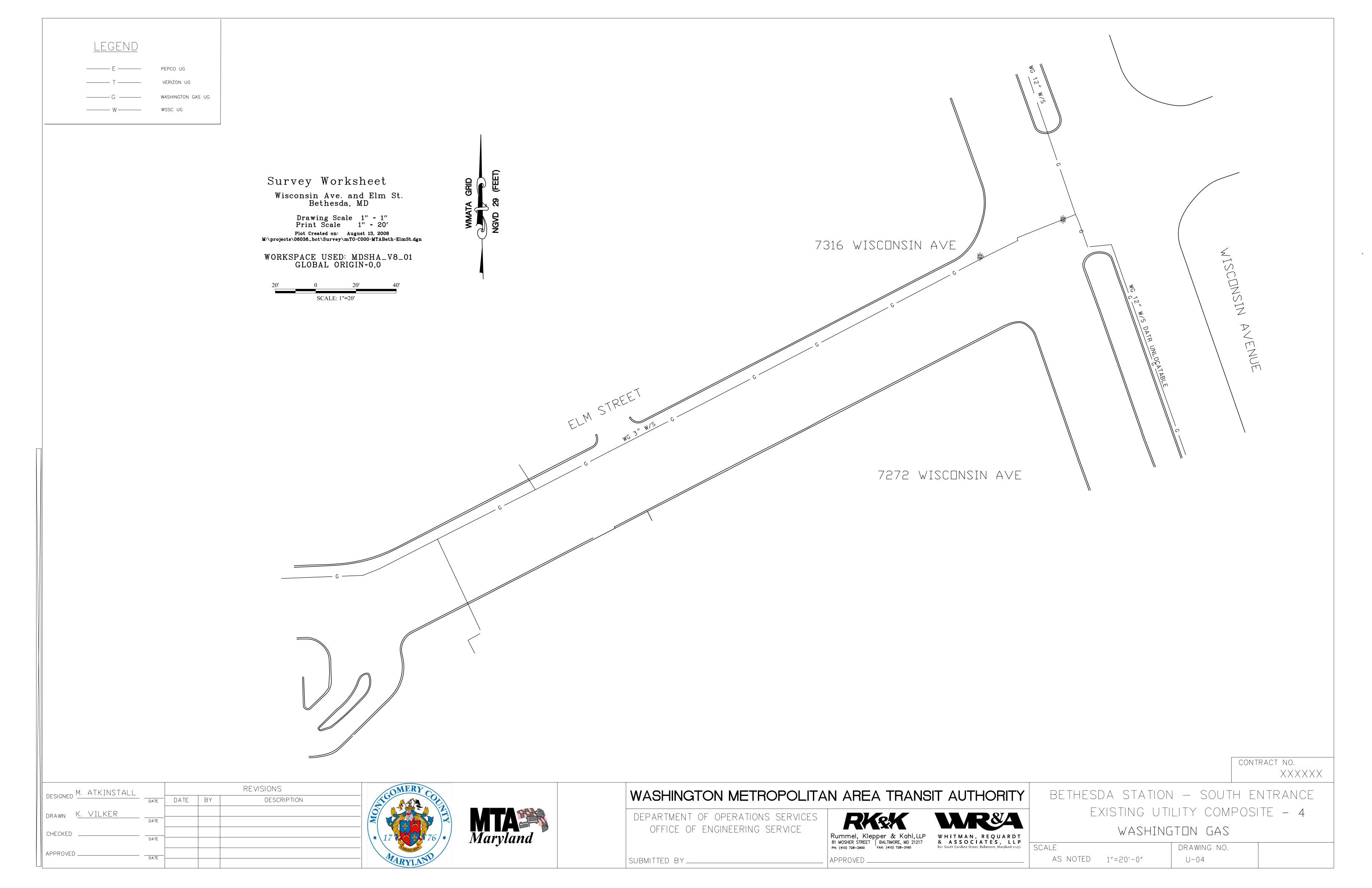
BETHESDA STATION - SOUTH ENTRANCE VERTICAL PROFILE — ELM STREET

SCALE	V: 1" = 2'	DRAWING NO.
	H: 1" = 20'	PP-UI









A. WMATA "GENERAL PROVISIONS AND STANDARD SPECIFICATIONS FOR CONTRACT DRAWINGS." B. WMATA "DIRECTIVE DRAWINGS."

2. DESIGN CRITERIA:

A. WMATA "MANUAL OF DESIGN CRITERIA FOR MAINTAINING AND CONTINUED OPERATION OF FACILITIES AND SYSTEMS, MAY 2008."

B. ACI 318-99 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ALTERNATE DESIGN METHOD.

C. AISC MANUAL OF STEEL CONSTRUCTION, 14TH EDITION.

D. AMERICAN WELDING SOCIETY STANDARD D1.1.

E. ASCE 7-10 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES.

F. INTERNATIONAL BUILDING CODE, 2012.

3. DESIGN LOADING

A. DEAD LOADS CONCRETE

150 PCF STEEL 490 PCF

- SOILS 130 PCF (BUOYANT WEIGHT = 68 PCF) - ROCK 170 PCF

B. LIVE LOADS

 STATION PLATFORMS 150 PSF

STAIRWAYS 150 PSF OR CONCENTRATED LOAD OF 300 LBS. ON STAIR TREAD

MEZZANINE PASSAGEWAYS

- EQUIPMENT AND SERVICE ROOMS 250 PSF (UNO) HORIZONTAL LOAD AT TOP OF 150 PLF

CONCRETE PARAPET - VERTICAL LOAD AT TOP OF

100 PLF CONCRETE PARAPET

SAFETY WALKS

200 LBS. IN ANY DIRECTION OR 50 PLF RAILINGS 70 PSF

- AIR PRESSURE FROM RUNNING TRAINS ON SERVICE AREA WALLS, DOORS & HARDWARE

- GRATINGS AND HATCHES 250 PSF

4. DESIGN LOADINGS FOR ELEVATORS

A. SURFACE ELEVATORS

SNOW LOAD 30 PSF BASIC WIND LOAD 40 PSF

 CANOPY FRAME LIVE LOAD 100 PLF FOR FREE EDGES

 ELEVATOR PIT SLAB AS PER MEP AND ELEVATOR MANUFACTURER

DWGS. PLUS IMPACT

150 PSF

150 PSF

85 PSF

B. MEZZANINE TO PLATFORM ELEVATORS

- ENCLOSURE, GLAZED AREA 30 PSF ENCLOSURE, UNGLAZED AREA 15 PSF

5. WIND LOADS

- COMPONENTS AND CLADDING IN ACCORDANCE WITH ASCE 7-10

 BASIC WIND VELOCITY 90 MPH EXPOSURE - IMPORTANCE FACTOR

CATEGORY

6. CONCRETE A. ALL CONSTRUCTION JOINTS IN EXTERIOR WALLS SHALL BE BONDED JOINTS.

B. ALL VERTICAL CONSTRUCTION JOINTS IN EXTERIOR WALLS AND SLABS SHALL BE KEYED.

C. ALL HORIZONTAL CONSTRUCTION JOINTS IN INTERIOR MEMBERS SHALL BE KEYED.

D. ALL VERTICAL CONSTRUCTION JOINTS IN INTERIOR MEMBERS SHALL BE KEYED.

E. ADDITIONAL HORIZONTAL AND VERTICAL CONSTRUCTION JOINTS MAY BE ADDED ONLY WITH WRITTEN AUTHORIZATION OF THE ENGINEER. ENGINEER APPROVED ADDITIONAL CONSTRUCTION JOINTS SHALL NOT RESULT IN ADDITIONAL EXPENSE TO THE OWNER.

F. ALL CONTRACTION JOINTS SHALL HAVE A BOND BREAKER APPLIED.

G. PROVIDE 9" PVC DUMBBELL WATERSTOPS IN ALL EXTERIOR CONSTRUCTION AND CONTRACTION JOINTS. FOR JOINT DETAILS, SEE DWG. NO. ST-S-001.

H. CHAMFER ALL EXPOSED EDGES 3/4" X 3/4". CHAMFER REQUIRED UNLESS NOTED OTHERWISE IN DRAWINGS.

I. ALLOW 48 HOURS MINIMUM CURING TIME BETWEEN PLACEMENT OF ADJACENT CONCRETE POURS.

J. ALL CONDUITS IN THE FINAL LINER AND SLABS SHALL BE ROUTED ON THE DRY SIDE OF THE WATER STOPS.

f'c = 5,000 PSI

K. REINFORCED CONCRETE STRUCTURES SHALL BE DETAILED AND CONSTRUCTED IN ACCORDANCE WITH THE CURRENT "ACI STANDARD BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318).

L. SEE ARCHITECTURAL, CIVIL, MECHANICAL AND ELECTRICAL DRAWINGS FOR ALL EMBEDDED ITEMS SUCH AS SCREWS, ANCHORS, ELECTRICAL CONDUITS, OPENINGS, ETC. WHICH MAY INTERFERE WITH CONCRETE CONSTRUCTION.

M. CONCRETE STRENGTH SHALL BE AS FOLLOWS:

PRECAST CONCRETE

- WALLS, SLABS, SLABS ON GRADE, BEAMS AND COLUMNS f'c = 4,000 PSI

 ALL OTHER CAST—IN—PLACE CONCRETE f'c = 3,500 PSI 7. REINFORCING STEEL

A. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60.

B. CONCRETE COVER FOR REINFORCING SHALL BE AS FOLLOWS.

UNLESS NOTED OTHERWISE ON THE DRAWINGS:

- UNFORMED CONCRETE BOTTOM BARS IN FOOTINGS AND SLABS ON EARTH, GRAVEL OR CRUSHED STONE......3"

- EXTERIOR UNFORMED SURFACE OF WALLS.......3"

- BEAMS, COLUMNS, SLABS AND WALLS EXPOSED TO GROUND OR WEATHER AFTER THE REMOVAL OF FORMS......2"

- BEAMS. COLUMNS AND WALLS NOT EXPOSED TO GROUND OR WEATHER AFTER THE REMOVAL OF FORMS....... 1/2"

- SLABS NOT EXPOSED TO GROUND OR WEATHER AFTER THE REMOVAL OF FORMS......3/4"

C. ALL SPLICES SHALL BE CLASS B TENSION LAPS UNLESS OTHERWISE NOTED ON THE PLANS.

D. ALL REINFORCEMENT SHALL BE MADE ELECTRICALLY CONTINUOUS. THIS INCLUDES INTERFACE WITH ADJACENT CONTRACTS; CHIP OUT EXISTING CONCRETE TO MAKE CONNECTION WHERE NECESSARY. UNLESS OTHERWISE SHOWN OR NOTED, ELECTRICALLY BONDED CIRCUMFERENTIAL CONTRACTION JOINTS ARE TO BE CONSTRUCTED AT A MAXIMUM INTERVAL OF 50'-0" MEASURED HORIZONTALLY. FOR DETAILS OF ELECTRICAL BONDING, SEE DWG. NO. ST-S-007 AND ST-S-021.

E. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI 315-99 "MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES." THE CONTRACTOR SHALL SUBMIT DRAWINGS OF REINFORCING STEEL BEFORE PROCEEDING WITH FABRICATION.

F. WELDED STEEL WIRE FABRIC SHALL CONFORM TO ASTM A185-06. THE FABRIC SHALL BE FURNISHED IN FLAT SHEETS.

G. UNLESS OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS, SPLICE AND EMBEDMENT LENGTHS FOR REINFORCING BARS SHALL BE IN ACCORDANCE WITH TABLE SHOWN BELOW:

RE	INFORCING	SPLICE AN	D DEVELOF	PMENT LEN	IGTHS (INC	HES)			
	#3	#4	#5	#6	#7	#8	#9	#10	#11
Ld	12	14	17	26	41	53	68	86	106
TOP BAR Ld	14	18	22	36	46	60	77	97	120
Ls	14	18	22	33	53	69	88	112	138
TOP BAR Ls	17	23	29	47	60	78	100	127	156

Ld=DEVELOPMENT LENGTH

Ls=SPLICE LENGTH

NOTE: TOP BARS ARE DEFINED AS HAVING MORE THAN 12" OF FRESH CONCRETE CAST BELOW BAR.

8. STRUCTURAL STEEL

A. MATERIALS SHALL CONFORM TO THE FOLLOWING:

ASTM A572 GRADE 50 W—SHAPES SHAPES AND PLATES ASTM A36

TUBFS ASTM A500 GRADE B STRUCTURAL BOLTS ASTM A325 ANCHOR BOLTS ASTM F1554 GRADE 55

ASTM F432 ROCK BOLTS B. THE CONTRACTOR SHALL SUBMIT ERECTION PLANS AND SHOP DETAILS BEFORE PROCEEDING

C. MILL BOTTOM OF ALL COLUMNS AND FINISH TOP OF ALL BASE PLATES IN ACCORDANCE WITH AISC SPECIFICATIONS. BASE PLATES SHALL BE WELDED TO BOTTOM OF COLUMNS.

D. 1/4" THICK LEVELING PLATES SHALL BE USED UNDER ALL BEAMS AND COLUMNS RESTING ON CONCRETE.

E. ALL SHOP CONNECTIONS SHALL BE WELDED WITH ELECTRODES AS SPECIFIED. ALL FIELD CONNECTIONS SHALL BE HIGH STRENGTH BOLTED JOINTS, TYPE ST, EXCEPT WHERE NOTED. BOLTS SHALL BE A325 AND CERTIFIED AS NOT TO BE COUNTERFEIT.

F. ELECTRODES FOR WELDING CONNECTIONS SHALL BE AS FOLLOWS: SHIELDED METAL ARC E70XX

SUBMERGED ARC F7X-EXXX

G. CONNECTION DETAILS SHALL BE DESIGNED AND SUBMITTED ON SHOP DRAWINGS BY THE CONTRACTOR AND ACCOMPANIED BY COMPLETE STRUCTURAL CALCULATIONS PREPARED AND SIGNED AND SEALED BY AN ENGINEER, LICENSED IN THE STATE OF MARYLAND.

9. GENERAL REQUIREMENTS

A. ELEVATIONS ARE TO BE ACTUAL FINISH ELEVATION.

B. SHORING REQUIRED FOR THE STABILITY OF THE UNCOMPLETED STRUCTURE OR FOR INSTALLATION OR MODIFICATION OF STRUCTURAL MEMBERS SHALL BE THE CONTRACTOR'S RESPONSIBILITY. ANY REQUIRED TEMPORARY STRUCTURES SHALL BE DESIGNED FOR THE LOADINGS SHOWN ON DWG. NO. ST-S-009.

C. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS PRIOR TO THE START OF CONSTRUCTION. THE CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES OR CONFLIFTS FOUND IN CONTRACT DOCUMENTS AND/OR FIELD CONDITIONS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO SUPPLY THE ENGINEER WITH ALL FIELD DIMENSIONS REQUIRED TO CHECK DETAIL DRAWINGS. THE ± MARKS SHOWN WITH DIMENSIONS AND STATIONS DO NOT INDICATE ANY DEGREE OF PRECISION. THESE # MARKS INDICATE DIMENSIONS AND STATIONS FROM EXISTING PLANS THAT MAY VARY AND DO REQUIRE FIELD VERIFICATION BY THE CONTRACTOR.

D. CONTRACTOR SHALL COORDINATE ALL REQUIRED OPENINGS WITH MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS. CONTRACTOR SHALL COORDINATE FINAL SIZE AND LOCATION OF ALL OPENINGS WITH THE ACTUAL FOUIPMENT SUPPLIED, PROJECT REQUIREMENTS, AND WITH FIFLD CONDITIONS.

E. THE ENGINEER PERMITS NO ALTERATIONS OR OPENINGS THROUGH BEAMS OR COLUMNS. UNLESS DETAILED ON STRUCTURAL PLANS.

F. THE SIZES AND LOCATIONS OF EQUIPMENT PADS, PEDESTALS AND FLOOR AND SLAB OPENINGS ARE DEPENDENT ON THE ACTUAL EQUIPMENT FURNISHED. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE AND VERIFY ALL SUCH ITEMS. NO DIMENSIONS INDICATED ON THESE DRAWINGS SHALL BE ALTERED WITHOUT THE ENGINEERS APPROVAL.

G. THE STRUCTURES HAVE BEEN DESIGNED TO RESISTS DESIGN LOADS ONLY AS COMPLETED STRUCTURES, UNLESS NOTED OTHERWISE ON THE DRAWINGS. ANY PROPOSED APPLICATION OF CONSTRUCTION LOADS WHICH EXCEED THE DESIGN LOADS, OR ANY LOADS APPLIED TO A PARTIALLY COMPLETED STRUCTURE WILL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL THE COST OF ANALYSIS, REDESIGN AND ANY ADDITIONAL CONSTRUCTION COSTS RESULTING FROM THE REDESIGN SHALL BE ACCOMPLISHED AT THE CONTRACTOR'S EXPENSE.

H. WHERE A SPECIFIC MODEL AND/OR MANUFACTURER OF AN ITEM ARE NAMED ON A DRAWING AND/OR IN THE SPECIFICATIONS, THE MODEL AND/OR MANUFACTURER ARE THE BASIS OF DESIGN. ITEMS BY OTHER MANUFACTURERS OF EQUAL DESIGN MAY BE SUBMITTED TO THE ENGINEER FOR REVIEW AS APPROVED

I. ALL PLAN DIMENSIONS ON THE DRAWINGS ARE MEASURED IN A TRUE HORIZONTAL PLANE UNLESS NOTED OTHERWISE.

J. ALL VERTICAL DIMENSIONS SHALL BE MEASURED IN A TRUE VERTICAL PLANE FOR ALL STRUCTURES UNLESS NOTED OTHERWISE.

K. COLUMNS, WALLS, DOORS, CONSTRUCTION JOINTS AND ELEVATORS WITHIN THE STATION AND THE SERVICE AREAS SHALL BE PLACED TRULY VERTICAL, UNLESS NOTED OTHERWISE.

L. SUBMITTALS

- REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RESUBMITTAL AS SHOP DRAWINGS IS PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED.

- ALL FORMWORK, SHORING AND RESHORING SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER REGISTERED IN THE STATE OF MARYLAND. ALL SUBMISSIONS SHALL BEAR HIS SEAL AND SIGNATURE.

 ALL SHORING, SHEETING, AND DEWATERING SHALL BE THE TOTAL RESPONSIBILITY OF THE CONTRACTOR. SHEETING AND SHORING SHALL BE DESIGNED BY THE CONTRACTORS ENGINEER REGISTERED IN THE STATE OF MARYLAND. ALL SUBMITTALS SHALL BEAR HIS/HER SEAL AND SIGNATURE AND MUST ACCOUNT FOR THE CONSTRUCTION SEQUENCE, DRAINAGE AND WALL THICKNESS. SUPPORT OF EXCAVATION SYSTEM FOR SHAFT AND ARCH SHALL PROVIDE SUFFICIENT CLEARANCE FOR CONSTRUCTION EXCAVATION, DELIVERY OF EQUIPMENT AND MATERIALS, WORKER ACCESS AND CONSTRUCTION OF FINAL STRUCTURE.

- STEEL GRATING SHALL HAVE DEPTH AS SHOWN ON DRAWINGS. MANUFACTURER SHALL DESIGN GRATING FOR THE LOADS SPECIFIED IN SECTION "3 DESIGN LOADING". ALL GRATING SHALL BE GALVANIZED PER ASTM A123.

- CONTRACTOR SHALL FURNISH DIMENSIONED COORDINATED SHOP DRAWINGS AT ALL LEVELS SHOWING THE LOCATION OF ALL SLEEVES AND OPENINGS REQUIRED BY ALL TRADES ON ONE PLAN FOR EACH LEVEL. CONFLICTS BETWEEN TRADES WILL BE RESOLVED BY GENERAL CONTRACTOR BEFORE SUBMISSION TO THE ENGINEER.

- REVIEW OF SHOP DRAWINGS DESIGNED BY CONTRACTORS ENGINEER'S SHALL BE FOR GENERAL CONFORMANCE WITH THE PROJECT PARAMETERS AS INDICATED ON THE DRAWINGS AND IN THE GENERAL NOTES.

M. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO PROTECT ALL EXISTING STRUCTURES, CURBS, STREETS, ETC., FROM DAMAGE BY CONSTRUCTION EQUIPMENT. THE CONTRACTOR SHALL NOT DISPOSE OF ANY LIQUIDS, SLURRY, SOILS OR CHEMICALS ON THE SITE EXCEPT AS DIRECTED BY THE OWNER'S REPRESENTATIVE AND APPROVED BY THE DEPARTMENT OF ENVIRONMENTAL RESOURCES OR OTHER AGENCIES HAVING JURISDICTION.

N. THESE DESIGN STRUCTURAL DRAWINGS REPRESENT THE COMPLETED PROJECT, WHICH HAS BEEN DESIGNED FOR THE WEIGHTS OF THE MATERIALS INDICATED ON THE DRAWINGS AND FOR THE SUPERIMPOSED LOADS INDICATED IN THE DESIGN DATA. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS AND TO PROVIDE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGING, BRACING, SHEETING AND SHORING ETC.

O. ALL COSTS OF INVESTIGATION AND/OR REDESIGN, DUE TO CONTRACTOR MISLOCATION OF STRUCTURAL ELEMENTS OR OTHER LACK OF CONFORMANCE WITH THE PROJECT DOCUMENTS SHALL BE AT THE CONTRACTOR'S EXPENSE.

P. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR DETAILED INFORMATION REGARDING FINISHES, FIREPROOFING, ETC.

Q. IN CASE OF CONFLICT BETWEEN THE GENERAL NOTES, DETAILS AND SPECIFICATIONS, THE MOST RIGID REQUIREMENTS SHALL GOVERN.

R. BACKFILL PLACED AROUND SHAFT STRUCTURE SHALL BE SELECT MATERIAL.

10. EXCAVATION AND EARTHWORK

A. REFERENCE GEOTECHNICAL DATA REPORT FOR SOUTH ENTRANCE TO BETHESDA METRO STATION BY E2CR, INC. DATED FEBRUARY 2010 FOR INFORMATION.

B. LOCATE ANY EXISTNG UTILITY LINES OR APPURTENANCES AND ADVISE ENGINEER OF ANY CONFLICTS OR DISCREPANCIES SHOWN IN PLANS PRIOR TO CONSTRUCTION. DO NOT DEMOLISH ANY EXISTING STRUCTURES WITHOUT WRITTEN AUTHORIZATION.

C. ALL EXCAVATIONS SHALL BE KEPT DRY. STANDING WATER SHALL NOT BE ALLOWED IN EXCAVATIONS.

> CONTRACT NO. XXXXXX

REVISIONS DESIGNED ____ DESCRIPTION DATE DATE BY DRAWN E.M. THOMPSON CHECKED D.S. TUSING APPROVED _ DATE







DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE





STRUCTURAL GENERAL NOTES

BETHESDA STATION - SOUTH ENTRANCE

SCALE DRAWING NO. AS NOTED

SUBMITTED BY_

APPROVED ₋

ABBREVIATIONS

DRAWING

EQUAL

EACH FACE EACH WAY ELEVATION

EXPANSION

EXTERIOR

FINISHED

FOOTING

INBOUND

GALVANIZED

INSIDE FACE INTERIOR JOINT

KNOCK-OUT

LONG

POUNDS

MAXIMUM

MINIMUM

NUMBER

NOT OT SCALE

OUTSIDE FACE

OUTBOUND

OPENING

PLATE

RADIUS

SCHEDULE

STRUCTURAL

SYMMETRICAL

WIDE FLANGE WORKING POINT

STATION

THICK TOP OF TOP OF RAIL

TYPICAL

NUMBER (REINFORCING)

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

SERVICE WEIGHT CAST IRON

UNLESS NOTED OTHERWISE

MEZZANINE

HYDROSTATIC PRESSURE RELIEF

FEET

BOT.

BRG.

C.I.P.

CONST.

CONTR.

D.O.

DWG.

EQ.

EXP.

EXT.

FIN.

FT.

FTG. GALV. HPR

K.O.

LBS. MAX.

MEZZ. MIN.

Ν̈.Τ.S.

0.B.

O.F.

OPNG.

P.S.F.

P.S.I.

SCH.

STA.

SVCI

SYMM.

THK.

T/R TYP.

UNO

STRUCT.

EXISTING ANGLE APPROXIMATE CONTOUR BOTTOM INDEX CONTOUR BEARING CAST-IN-PLACE CENTERLINE CENTERLINE CHANNEL CLEAR, CLEARANCE CONCRETE, CONCRETE FILL CONSTRUCTION CONTRACTION DIAMETER DOOR OPENING

WATERSTOP REINFORCING STEEL W OR I SHAPE ANGLE SHAPE

PROPOSED

CENTERLINE 4 CONCRETE, CONCRETE FILL LIMITS OF DEMOLITION WATER BARRIER

OPENING

SYMBOLS

SECTION CUT SECTION DESIGNATION

DWG. NO. ON WHICH SECTION APPEARS

STRUCTURAL LEGEND

A S - 12

PLUS OR MINUS

1. THE ACTUAL CONSTRUCTION SEQUENCE AND SUPPORT OF EXCAVATION SYSTEM SHALL NOT RESULT IN LATERIAL PRESSURES TO THE SHAFT AND PASSAGEWAY ARCH RIBS HIGHER THAN THOSE SHOWN IN THE GEOTECHNICAL REPORT.

2. SEE GEOTECHNICAL DRAWINGS FOR SUPPORT OF EXCAVATION DETAILS BELOW EL. 280.00 ±. DESIGN OF SUPPORT OF EXCAVATION SYSTEM ABOVE EL. 280.00 ± SHALL BE BY CONTRACTOR. SYSTEM SHALL PROVIDE SUFFICIENT CLEARANCE FOR CONSTRUCTION EXCAVATION, DELIVERY OF EQUIPMENT AND MATERIALS AND WORKER ACCESS.

3. WATERPROOFING

APPROVAL.

A. WATERPROOFING MEMBRANE AND GEOTEXTILE SHALL SATISFY THE REQUIREMENTS OF THE SPECIFICATIONS.

B. WATERPROOFING ABOVE THE APPROXIMATE ROCK LINE SHALL SATISFY THE FOLLOWING REQUIREMENTS:

- APPLY WATERPROOFING MEMBRANE AND GEOTEXTILE TO ALL EXTERIOR CONCRETE SURFACES. FOR WATERPROOFING TERMINATION NEAR GROUND SURFACE, SEE DETAIL 3/S-25. - PROVIDE PROTECTIVE CONCRETE OVER WATERPROOFING ON ROOF

SLABS, SEE DETAIL 1/S-25. - WATERPROOFING SHALL BE SPLICED AT APPROXIMATE ROCK LINE,

SEE DETAIL 6/S-23. - BACKFILL VOID BETWEEN WATERPROOFING AND SUPPORT OF EXCAVATION WITH SELECT FILL.

C. WATERPROOFING DETAILS SHALL BE SUBMITTED TO THE ENGINEER FOR

CONTRACT NO.

XXXXXX

REVISIONS DESIGNED ____ DESCRIPTION DATE BY DATE DRAWN E.M. THOMPSON CHECKED D.S. TUSING APPROVED_





WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

SUBMITTED BY_

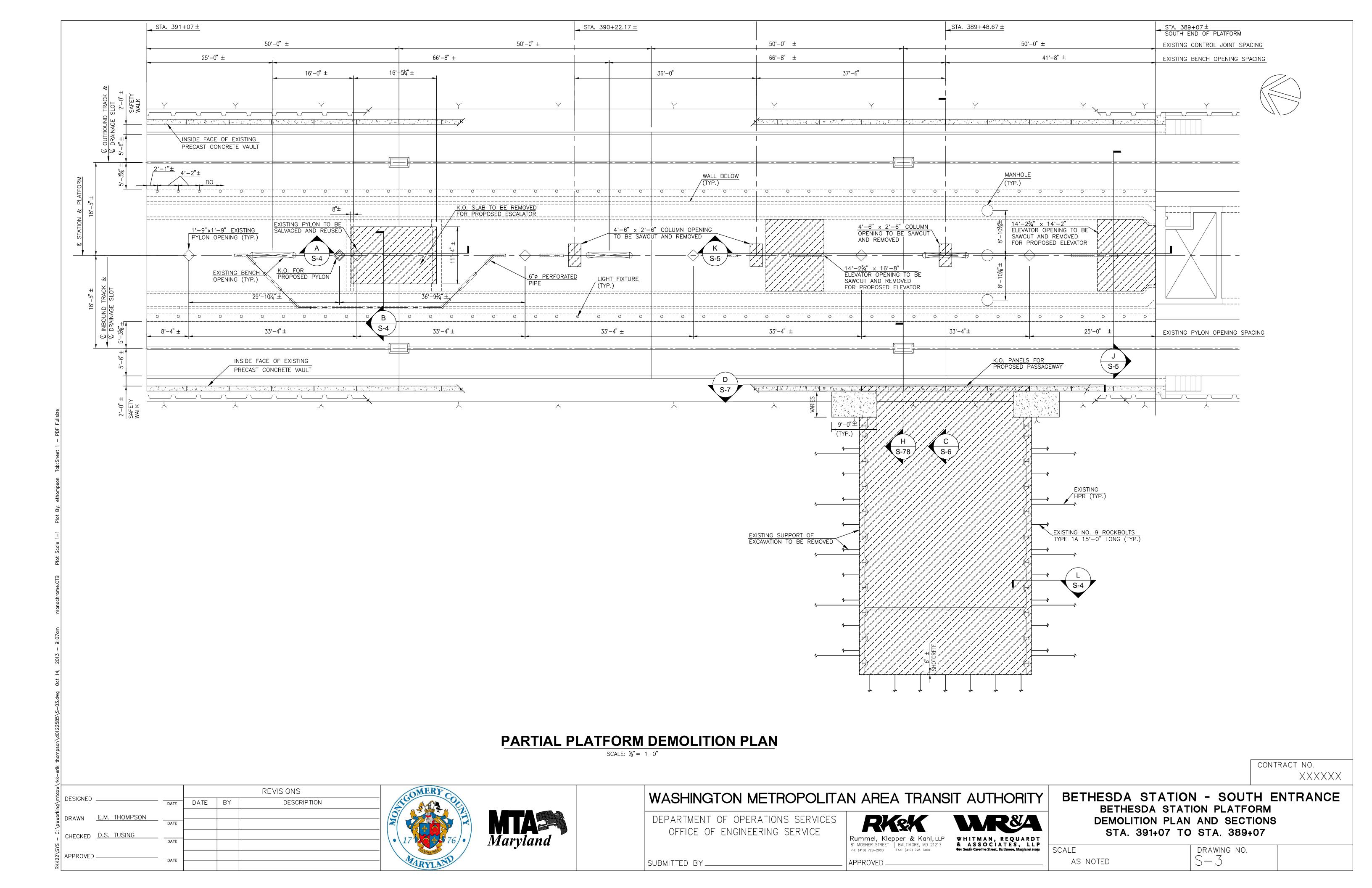
Rummel, Klepper & Kahl, LLP WHITMAN, REQUARDT 81 MOSHER STREET | BALTIMORE, MD 21217 PH: (410) 728–2900 FAX: (410) 728–3160

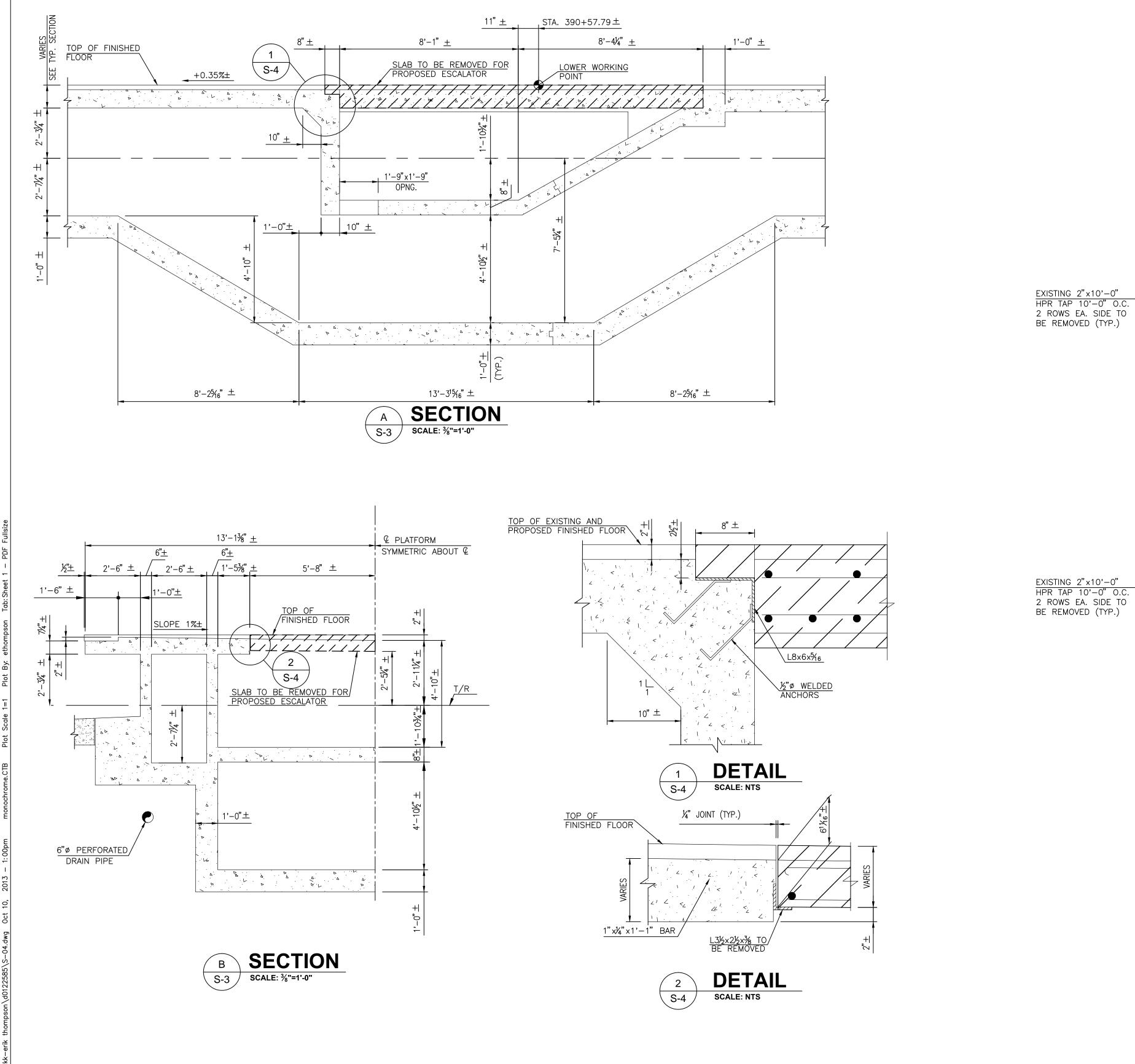
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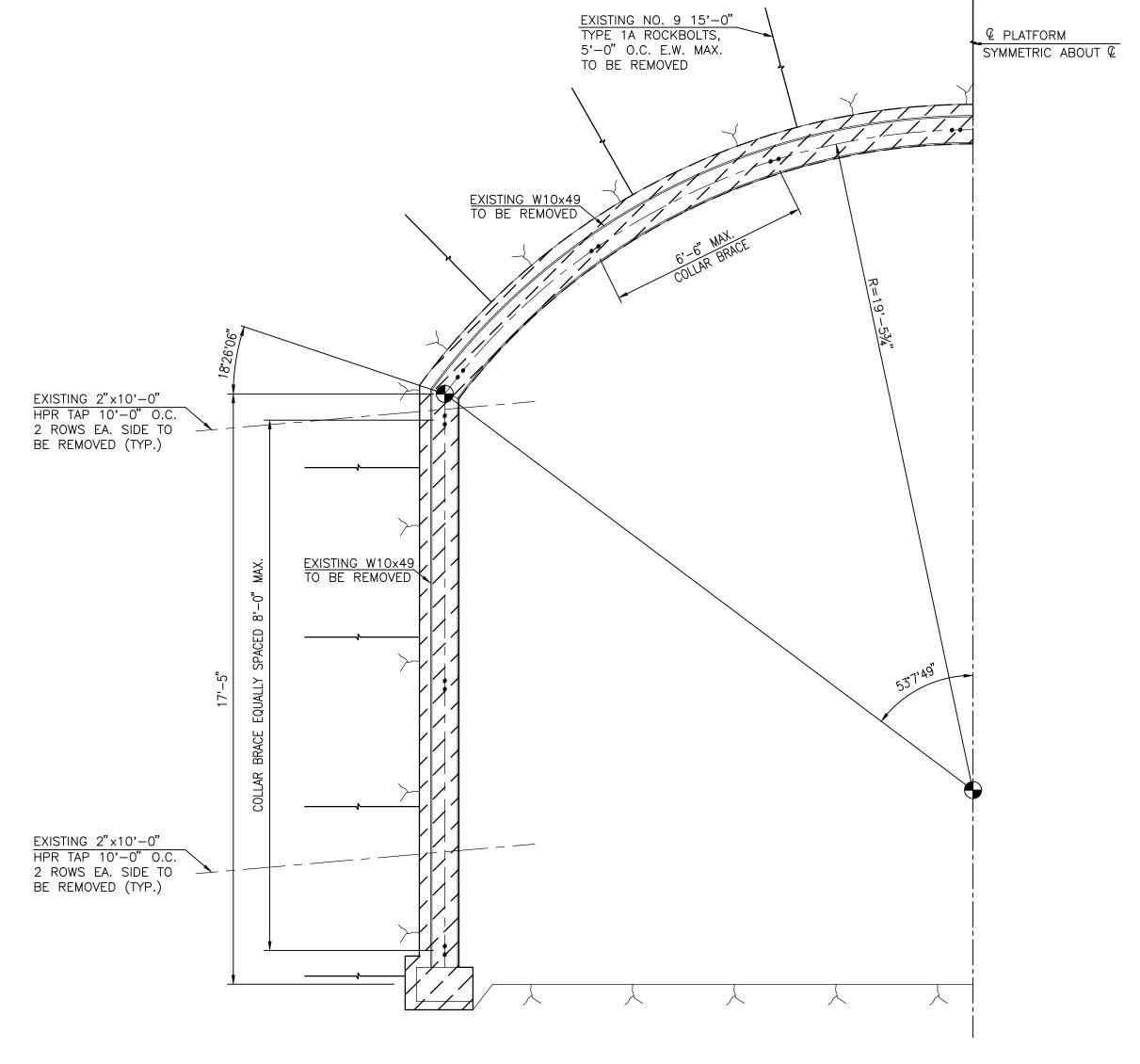
& ASSOCIATES, LLP

BETHESDA STATION - SOUTH ENTRANCE STRUCTURAL ABBREVIATION AND LEGEND

SCALE DRAWING NO. AS NOTED







SECTION

S-3 | SCALE: 3/8"=1'-0"

CONTRACT NO.

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DESIGNED

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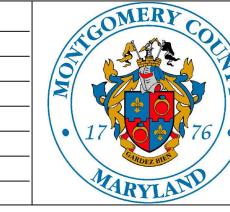
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OFFICE OF ENGINEERING SERVICE

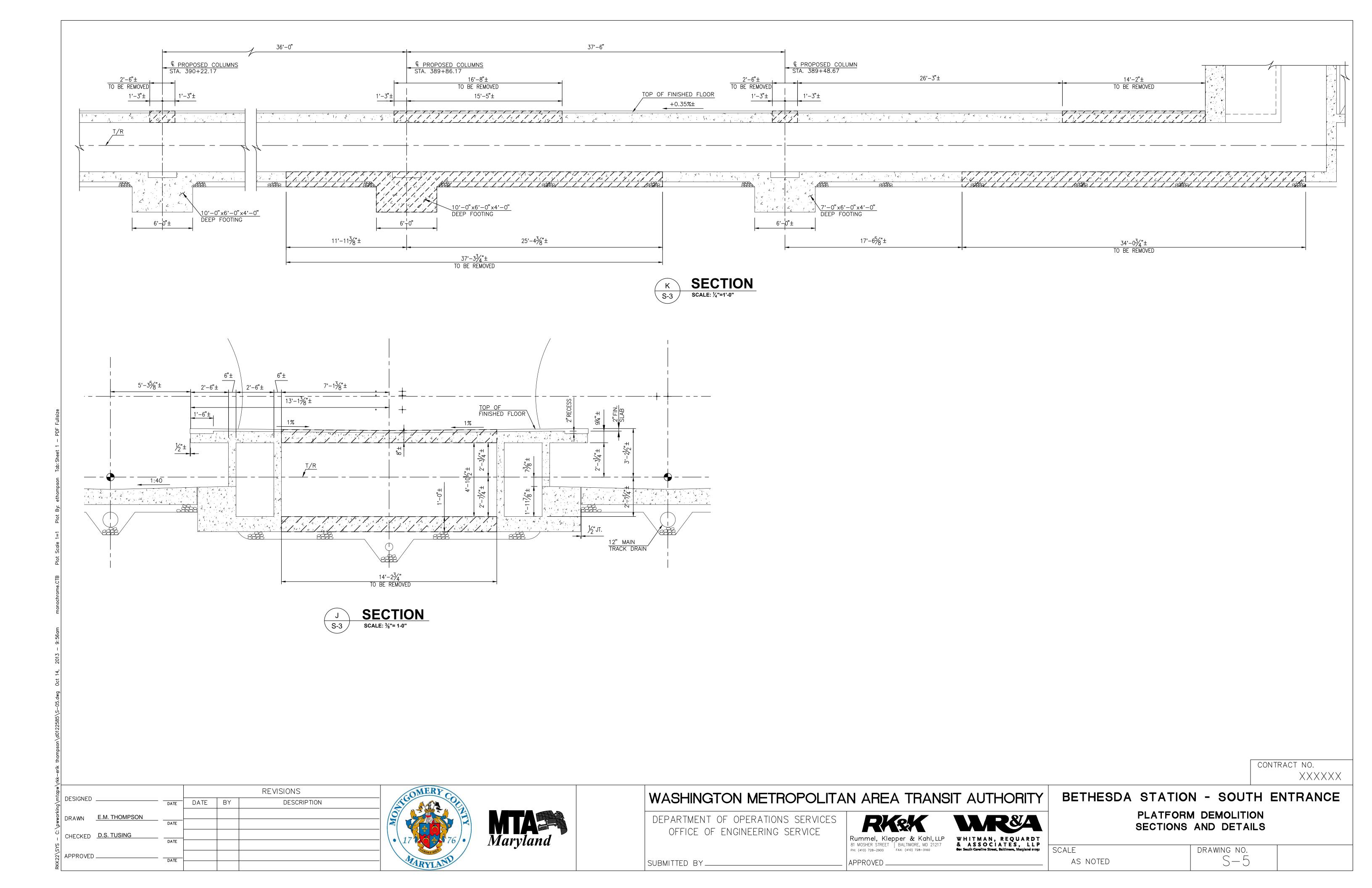
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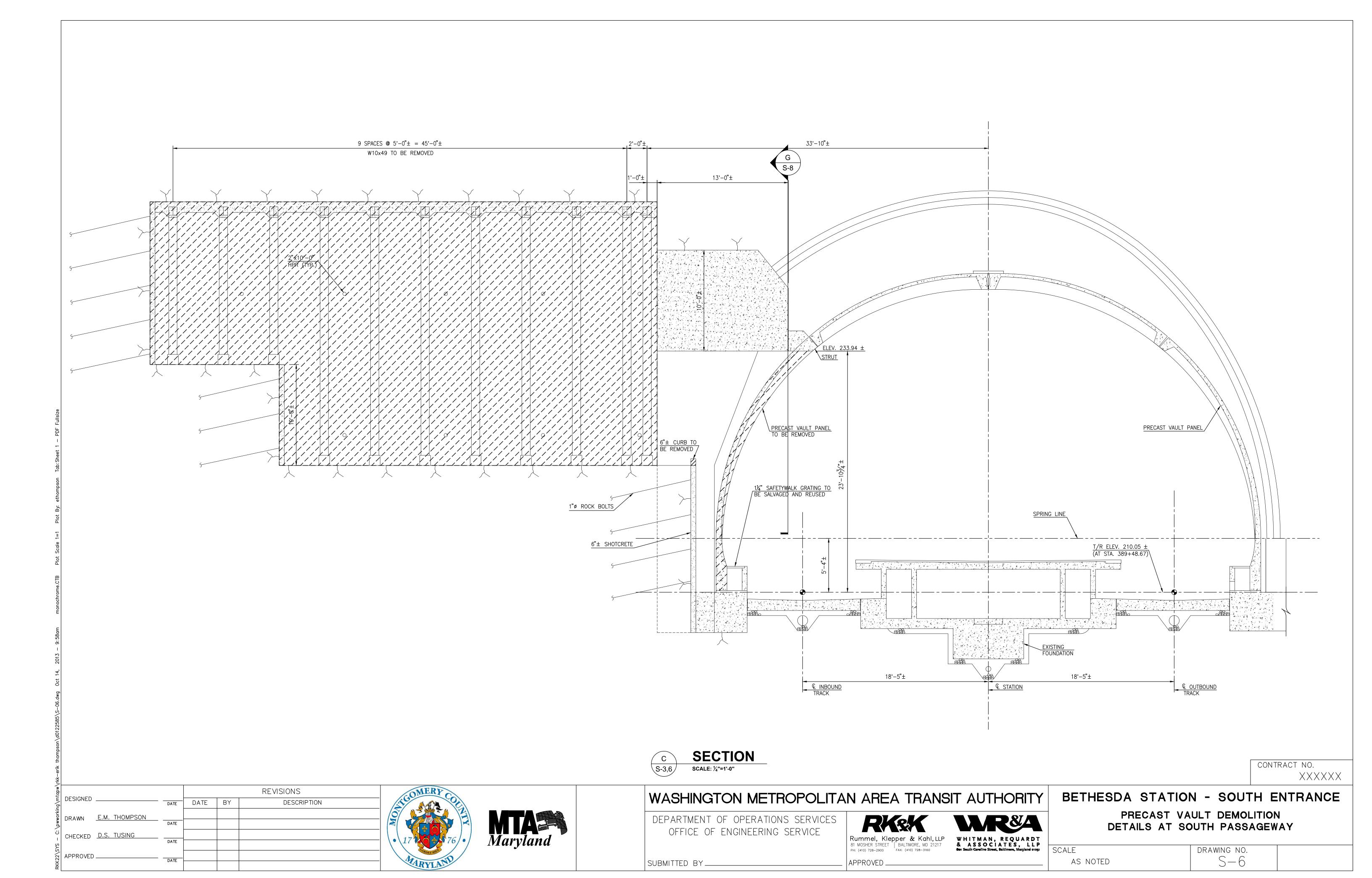
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81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728-2900 FAX: (410) 728-3160

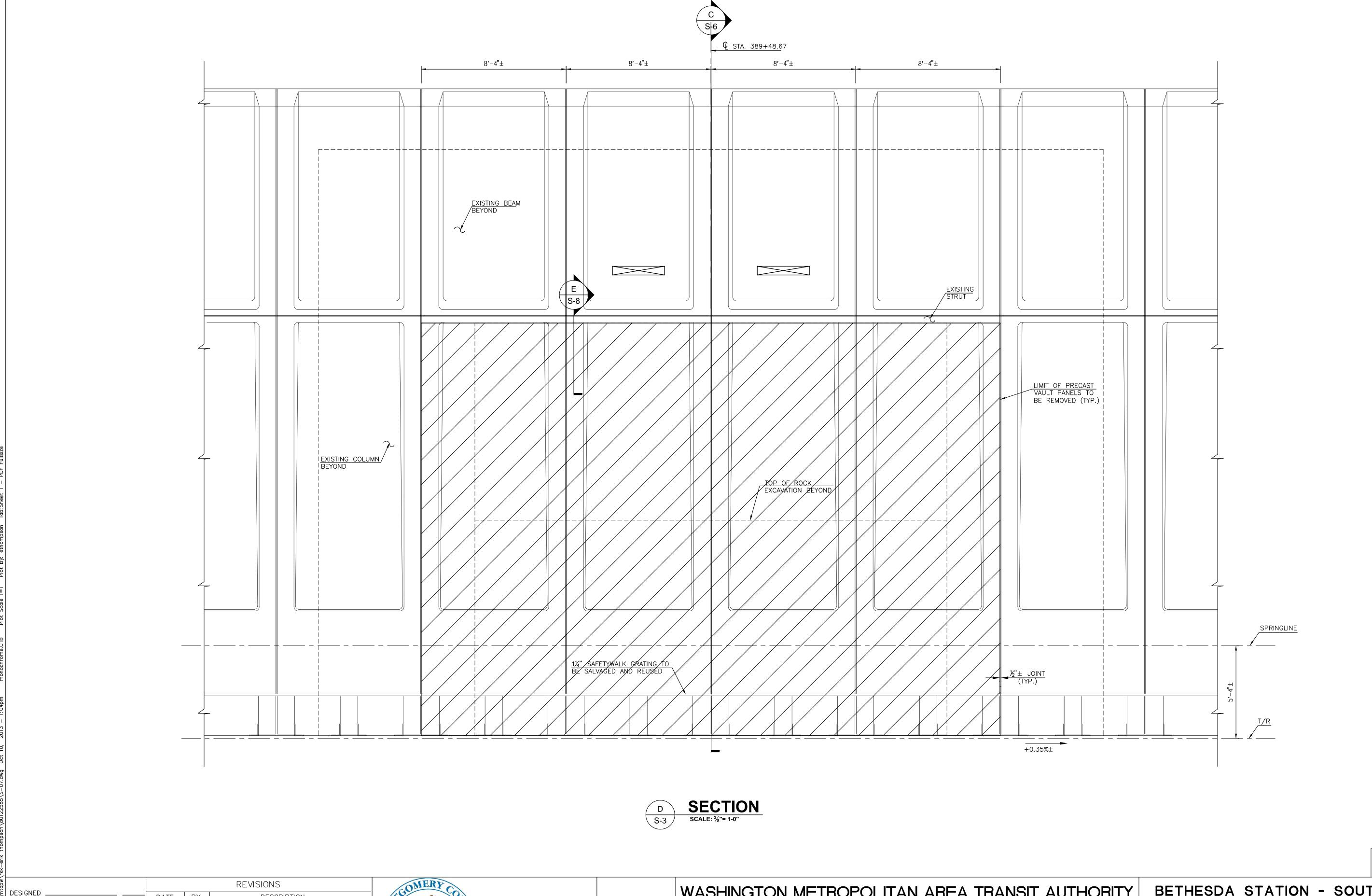
WHITMAN, REQUARDT & ASSOCIATES, LLP

BETHESDA STATION - SOUTH ENTRANCE BETHESDA STATION PLATFORM DEMOLITION PLAN AND SECTIONS STA. 391+07 TO STA. 389+07

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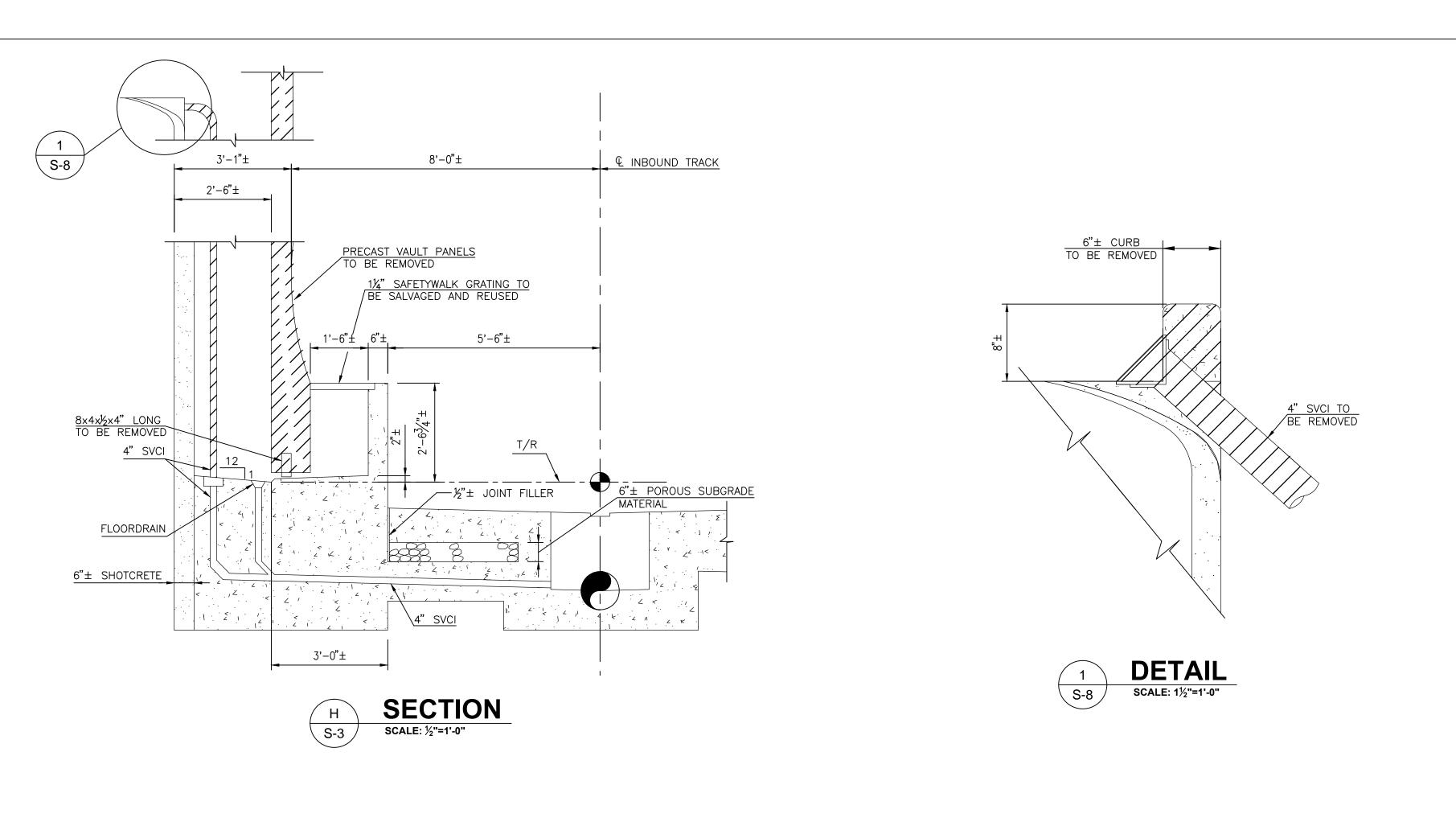
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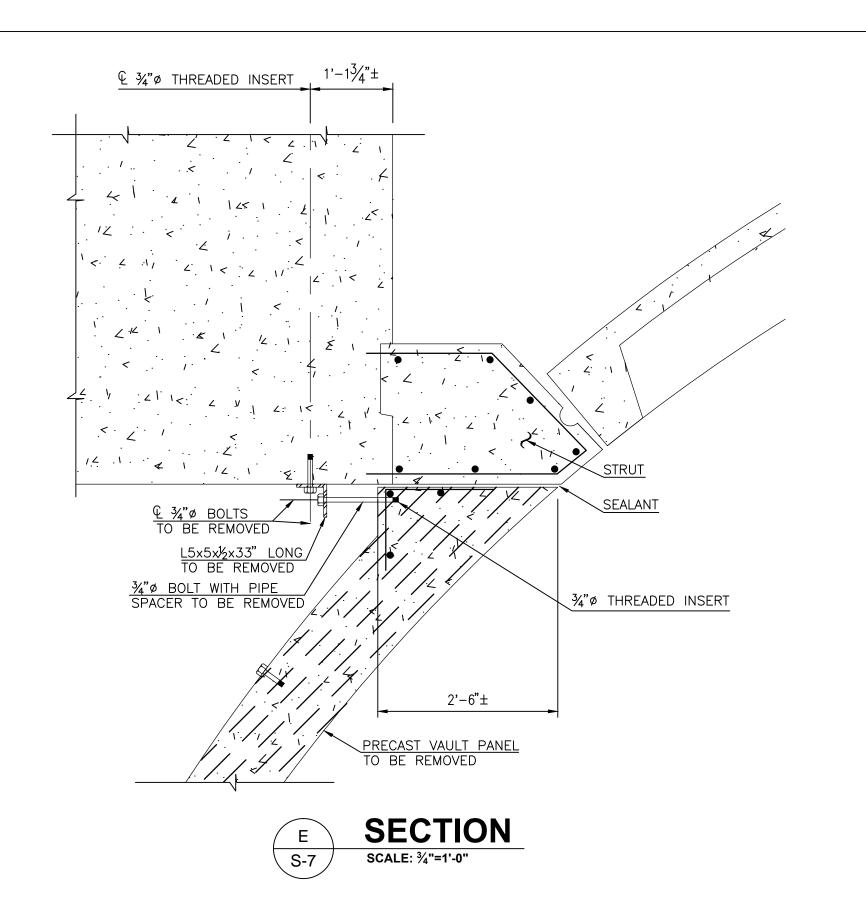
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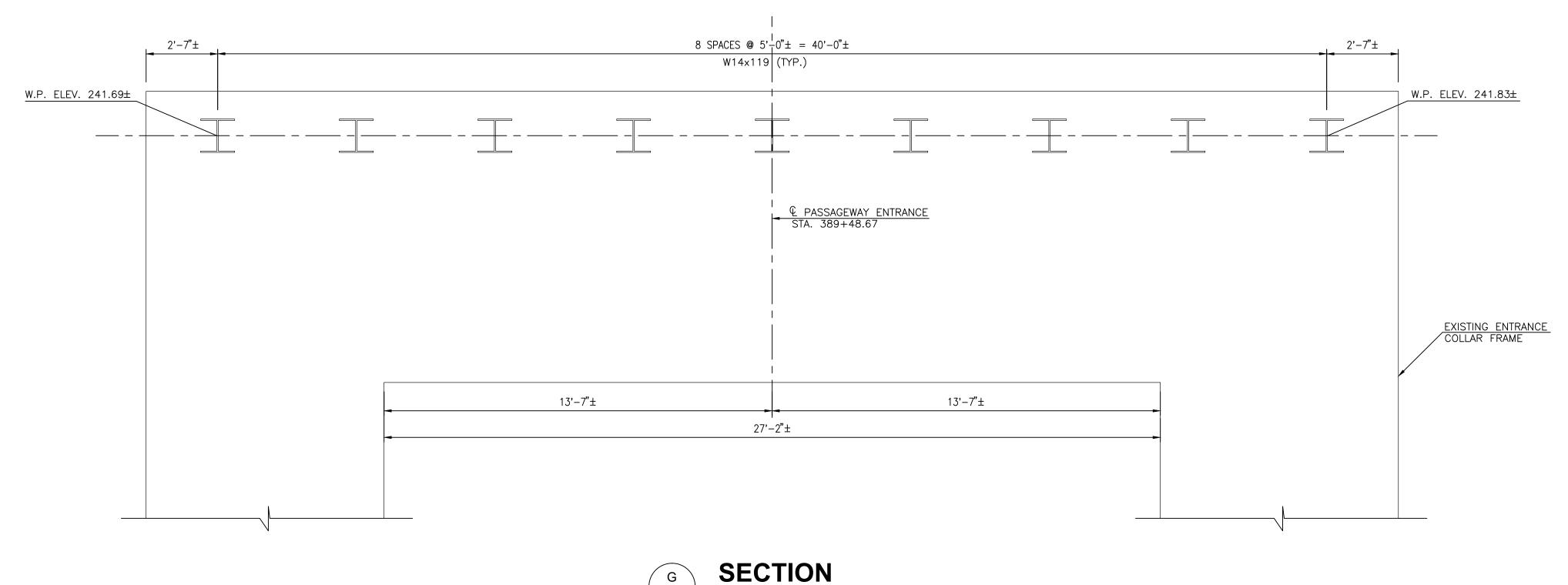
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81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728–2900 FAX: (410) 728–3160 WHITMAN, REQUARDT
& ASSOCIATES, LLP
Soil South Caroline Street, Baltimore, Maryland 21251

BETHESDA STATION - SOUTH ENTRANCE PRECAST VAULT DEMOLITION DETAILS AT SOUTH PASSAGEWAY

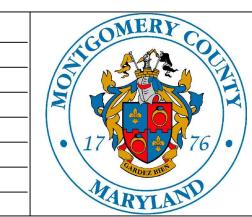
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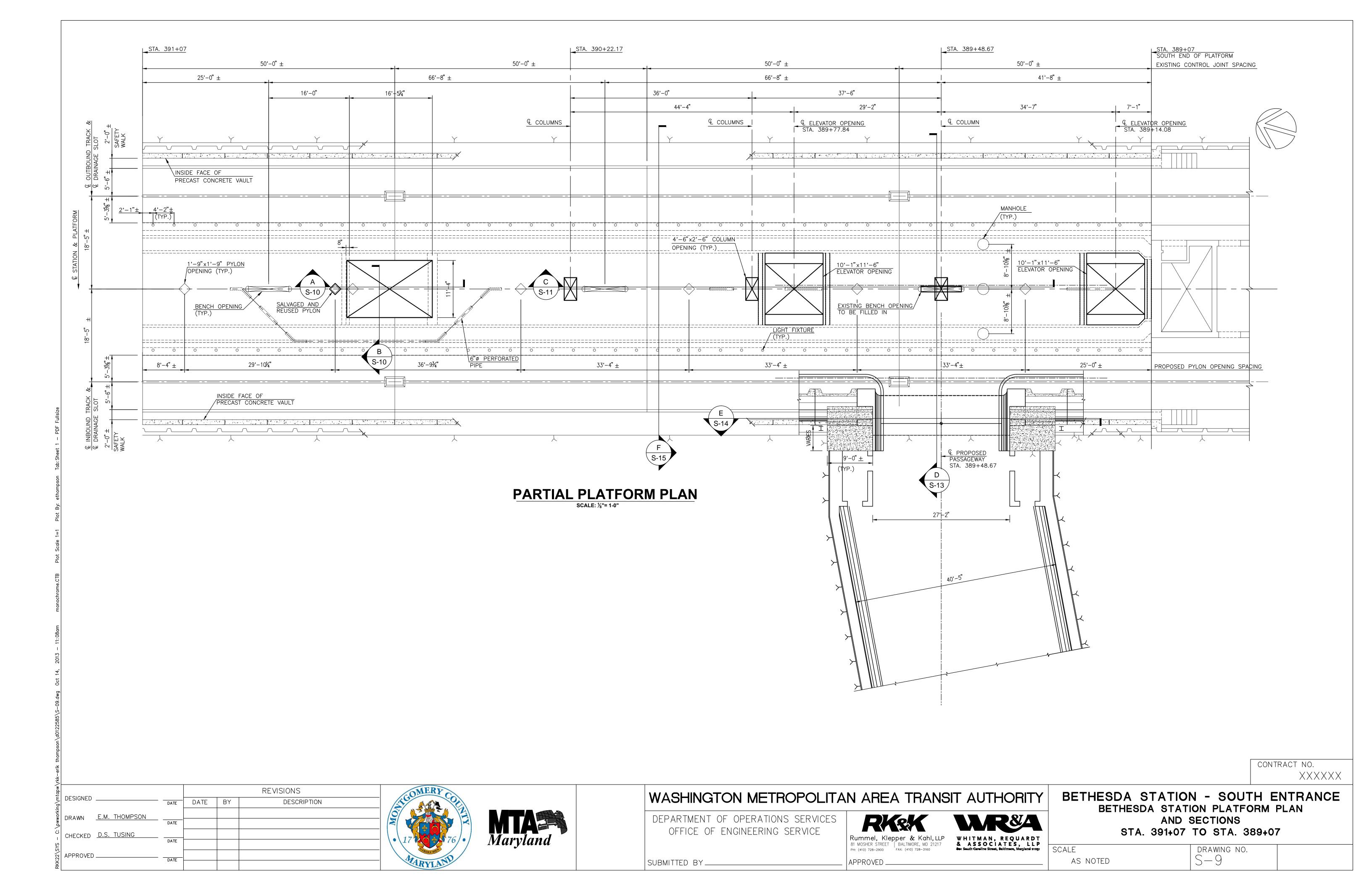
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81 MOSHER STREE
PH: (410) 728-2900

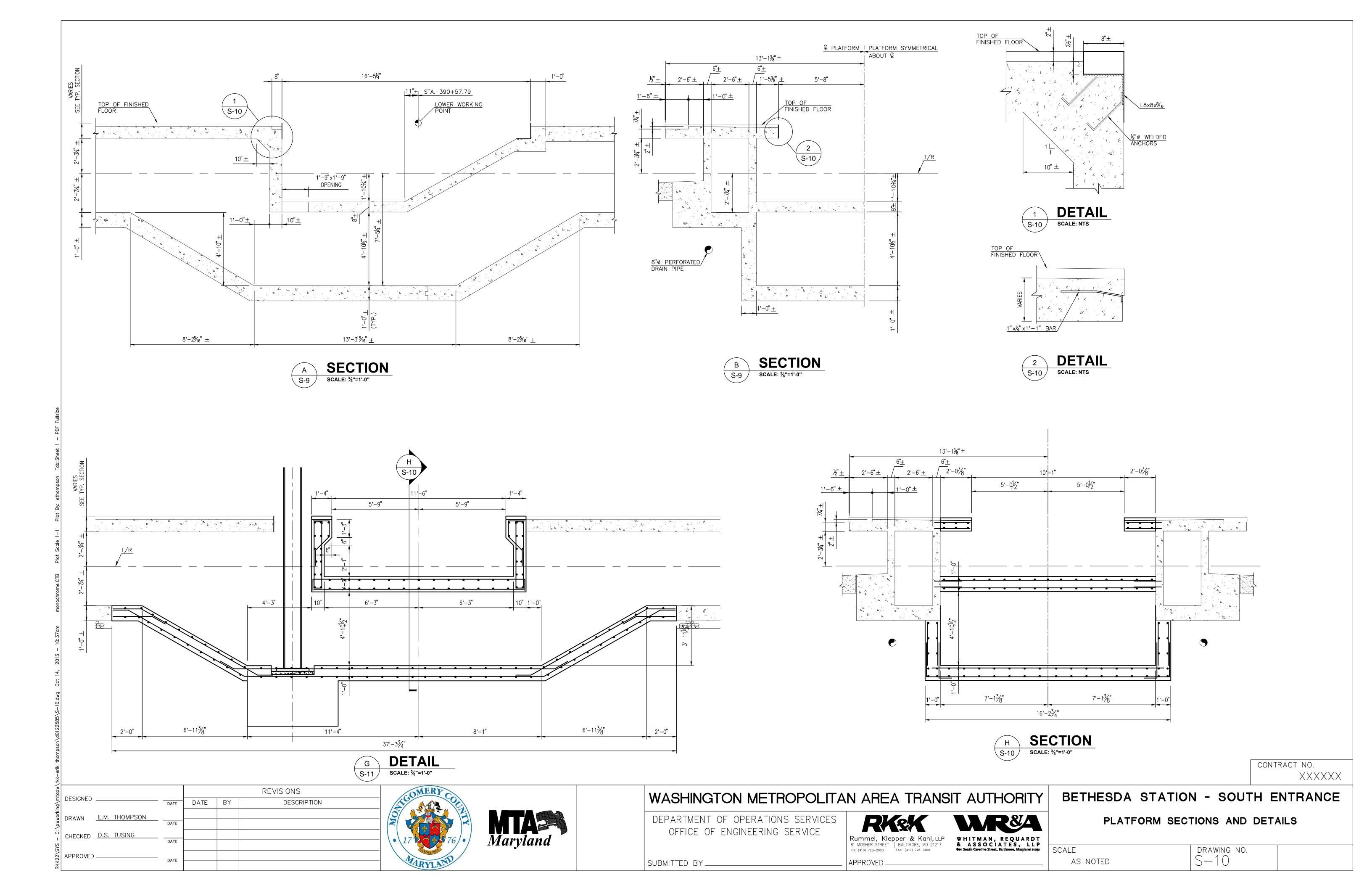
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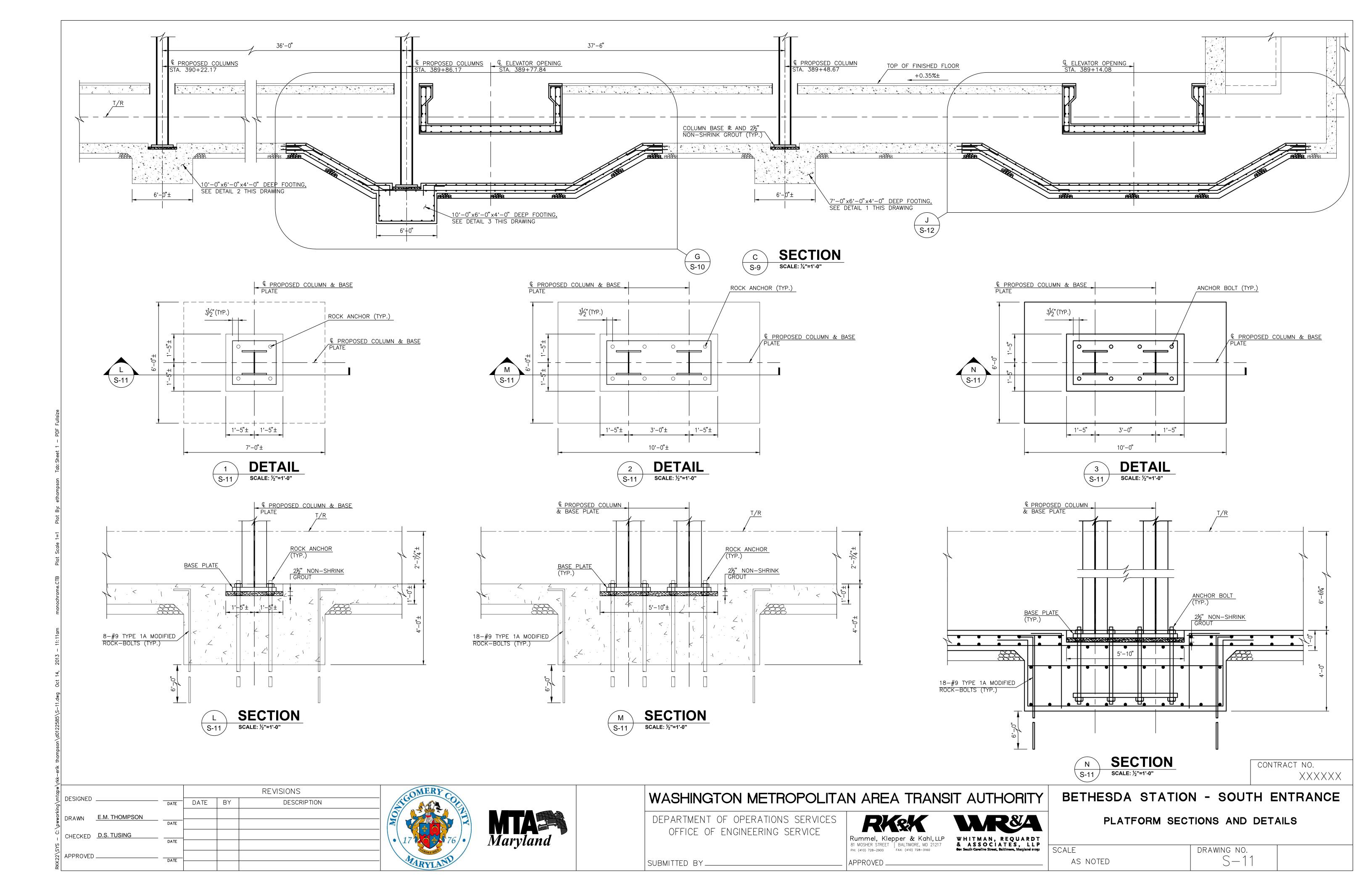
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Rummel, Klepper & Kahl, LLP 81 MOSHER STREET BALTIMORE, MD 21217 PH: (410) 728-2900 FAX: (410) 728-3160	WHITMAN, REQUARD & ASSOCIATES, LL Ser South Caroline Street, Baitimore, Maryland 21

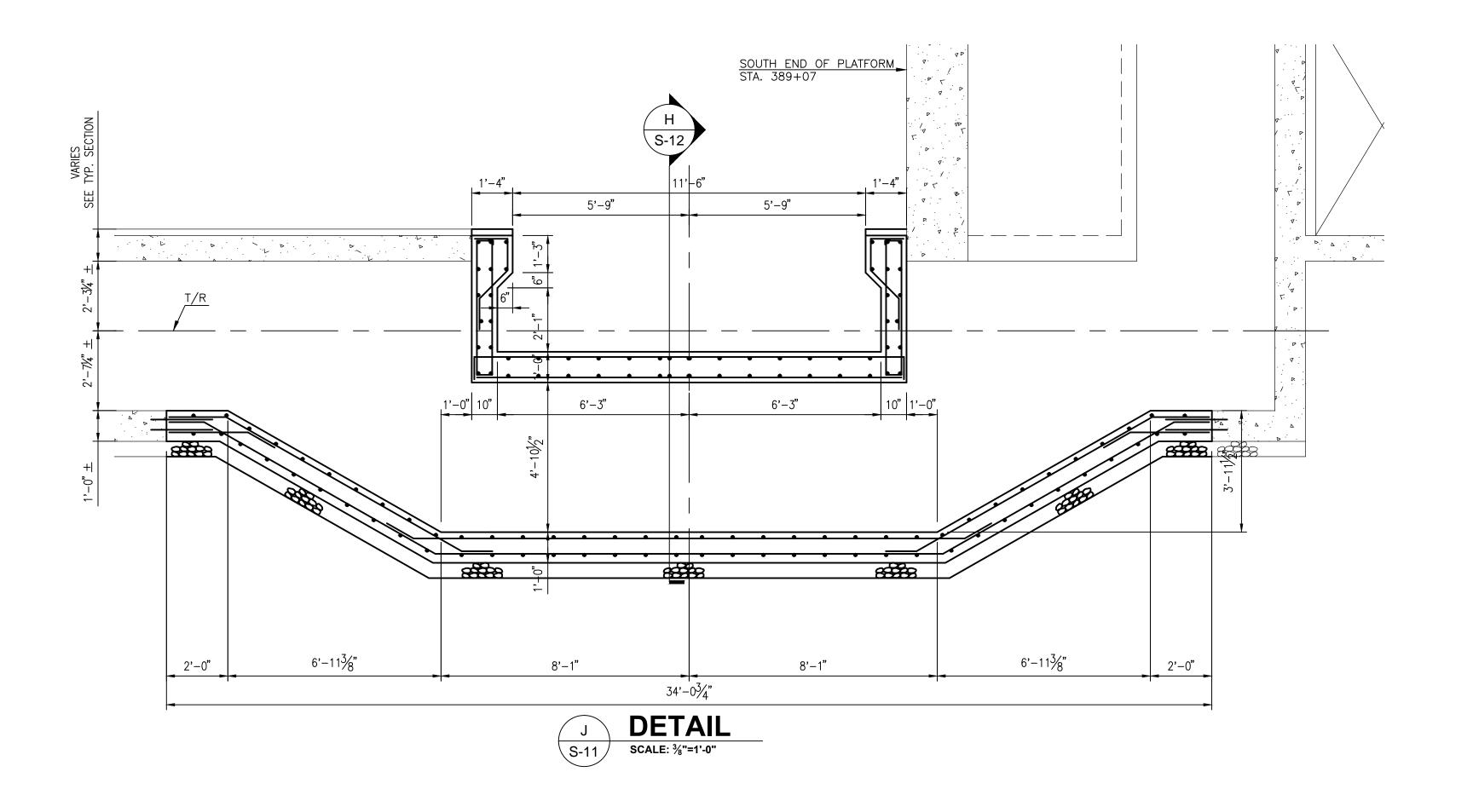
BETHESDA STATION - SOUTH ENTRANCE PRECAST VAULT DEMOLITION DETAILS AT SOUTH PASSAGEWAY

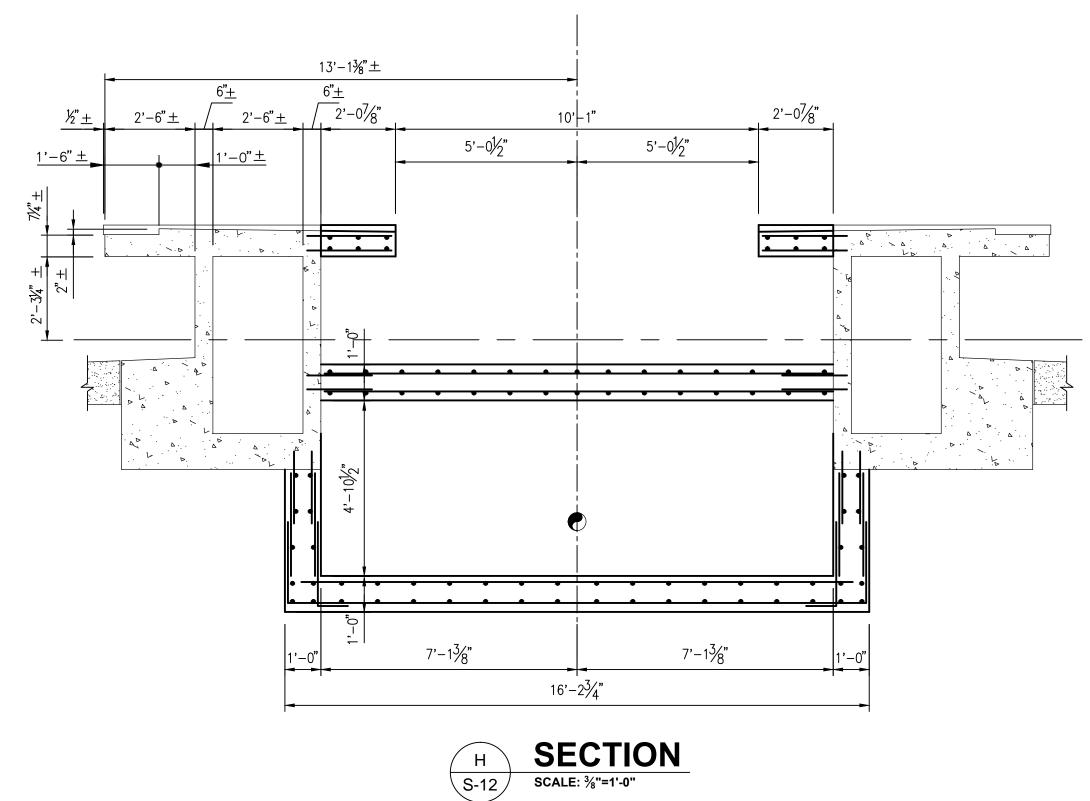
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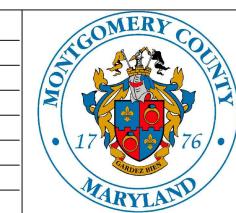
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REVISIONS

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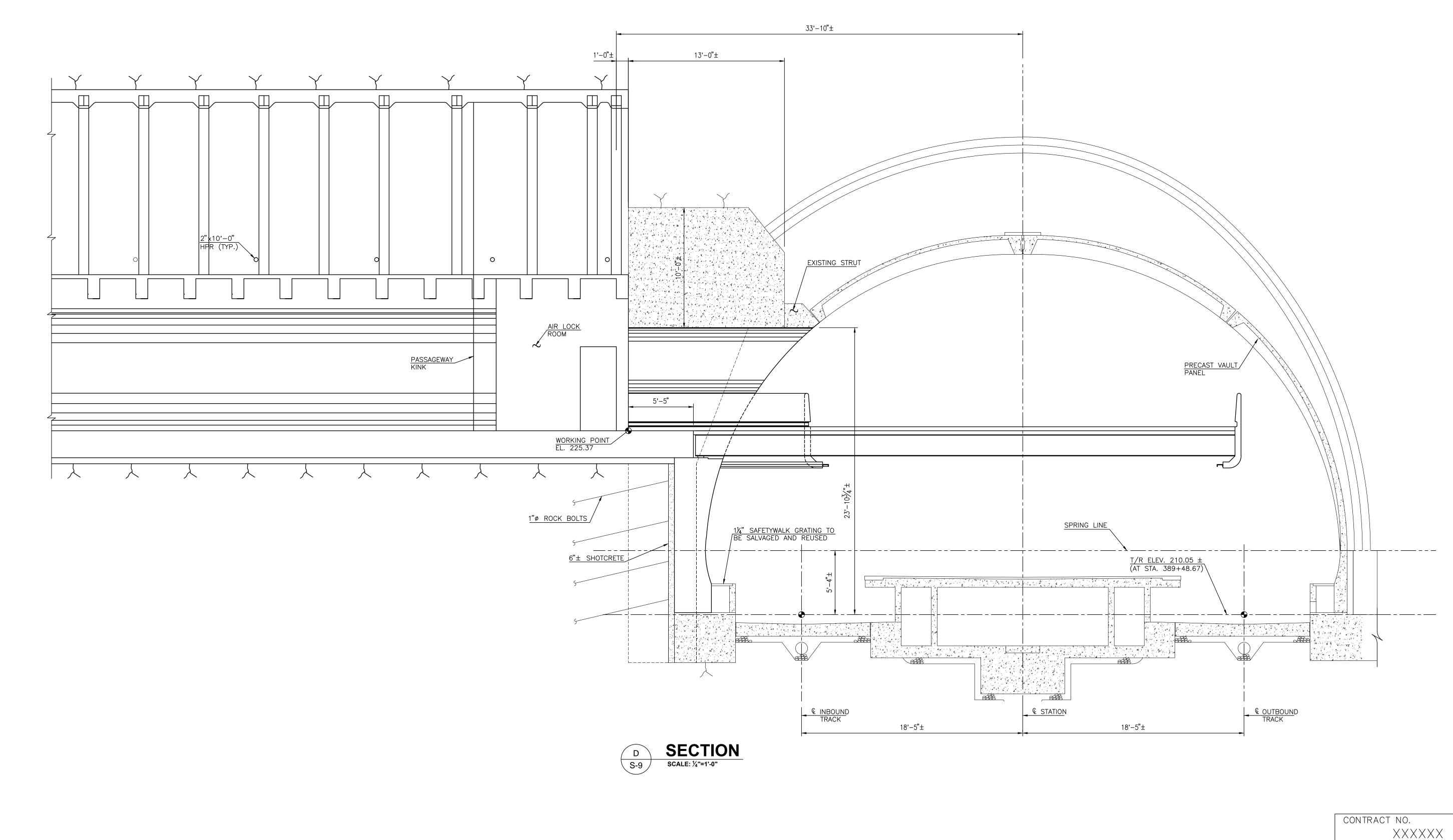
WHITMAN, REQUARDT & ASSOCIATES, LLP Soil South Caroline Street, Baltimore, Maryland 21251

BETHESDA STATION - SOUTH ENTRANCE

PLATFORM SECTIONS AND DETAILS

81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728–2900 | FAX: (410) 728–3160 | South Caroline Street, Baltimore, Maryland 21291 | SCALE

APPROVED _______ | AS NOTED | SCALE | SCALE | AS NOTED | S—12



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DATE





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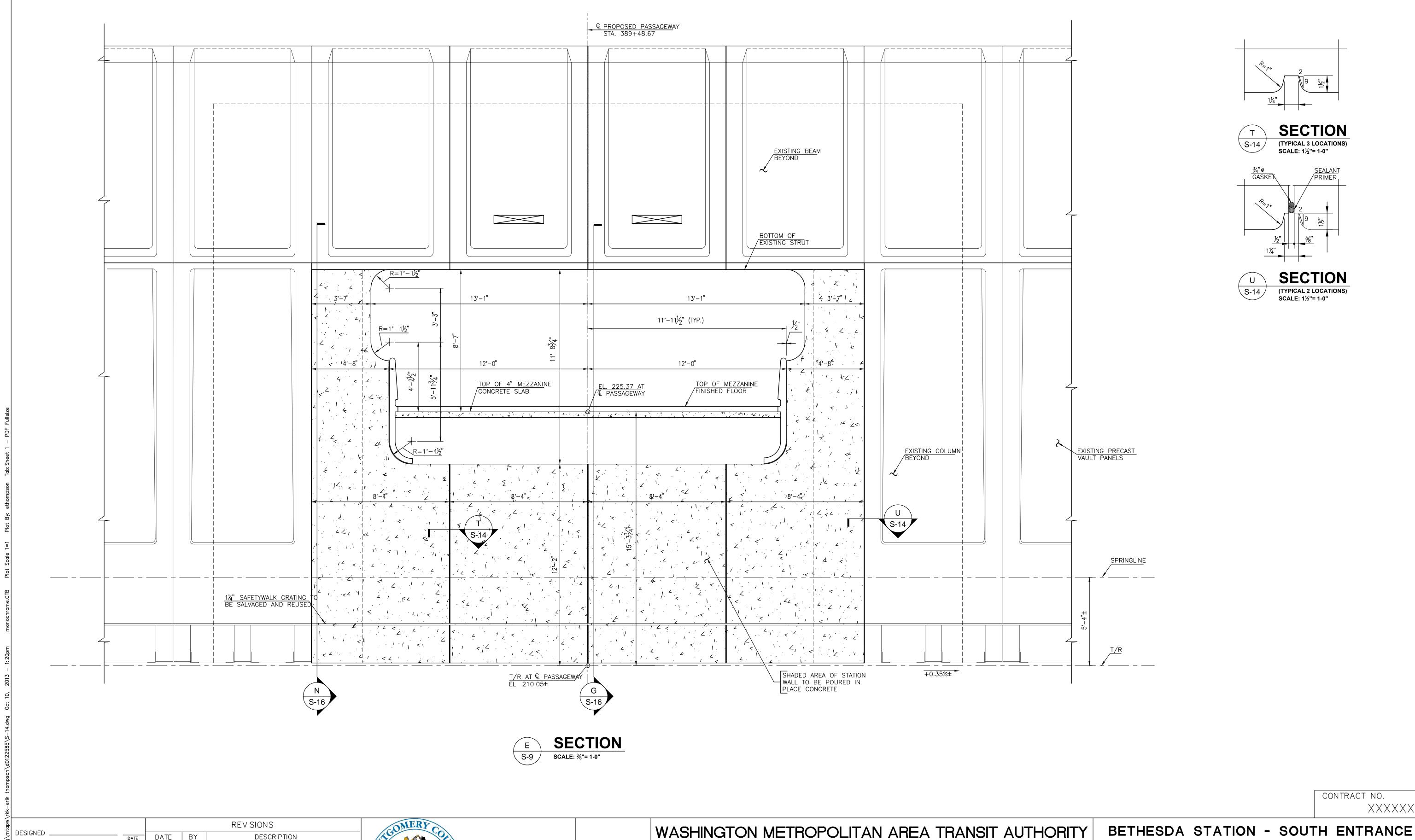
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BETHESDA STATION - SOUTH ENTRANCE

SECTION AT SOUTH PASSAGEWAY

SCALE DRAWING NO. S—13



BETHESDA STATION - SOUTH ENTRANCE VAULT DETAILS AT SOUTH PASSAGEWAY

SCALE DRAWING NO. S - 14

AS NOTED

SUBMITTED BY_

DEPARTMENT OF OPERATIONS SERVICES

Maryland

Maryland

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APPROVED _

DRAWN E.M. THOMPSON DATE

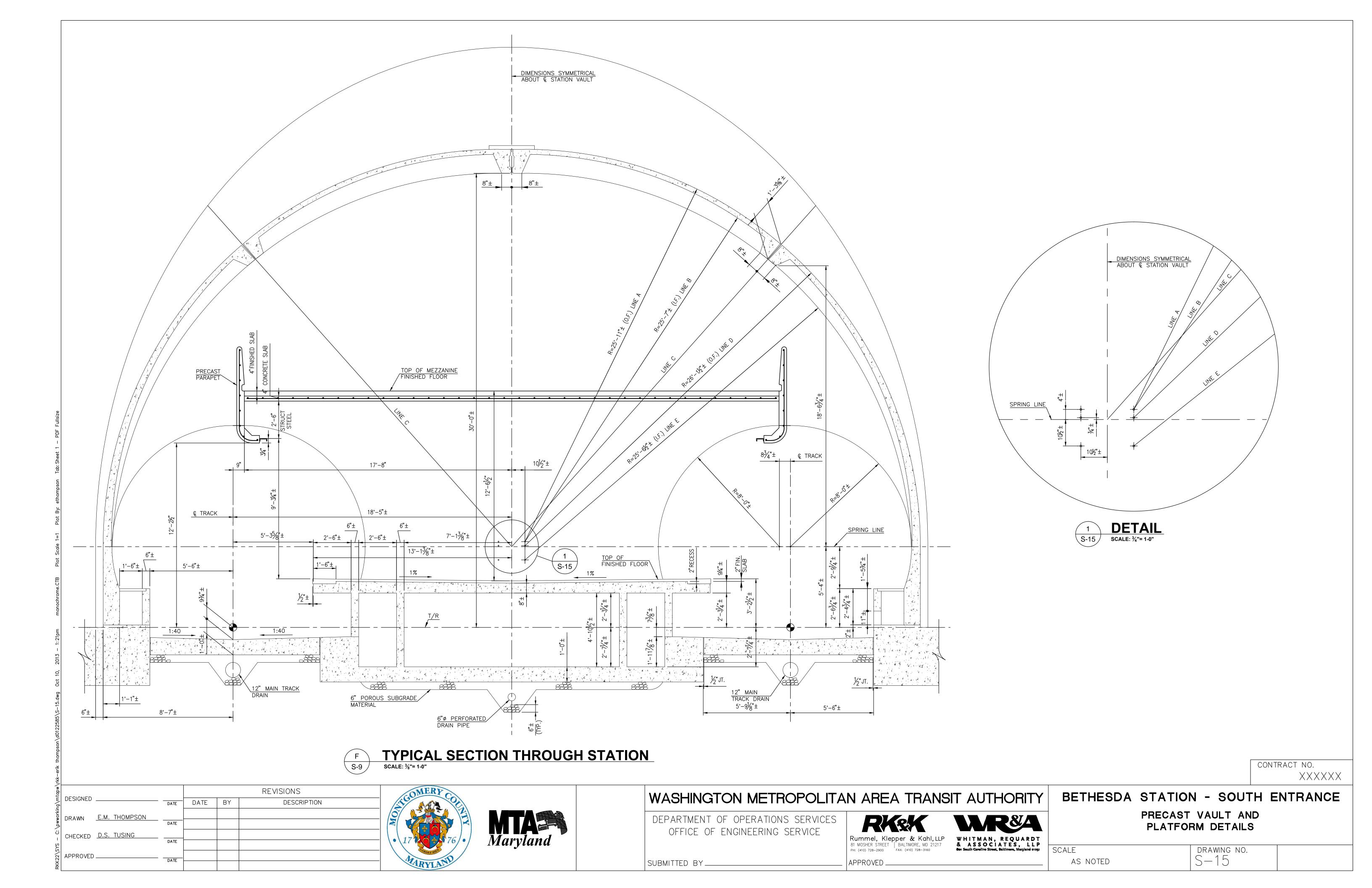
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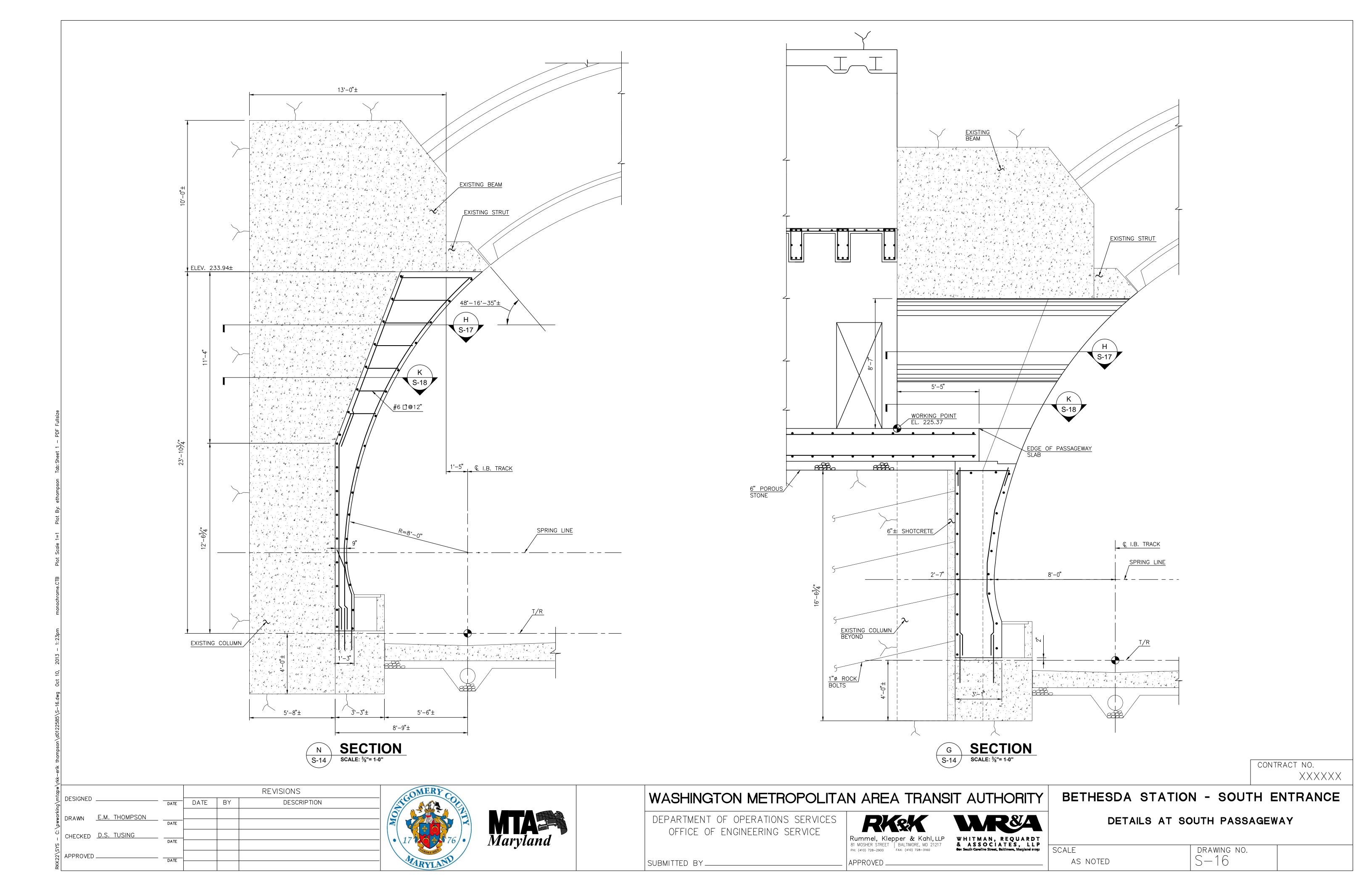
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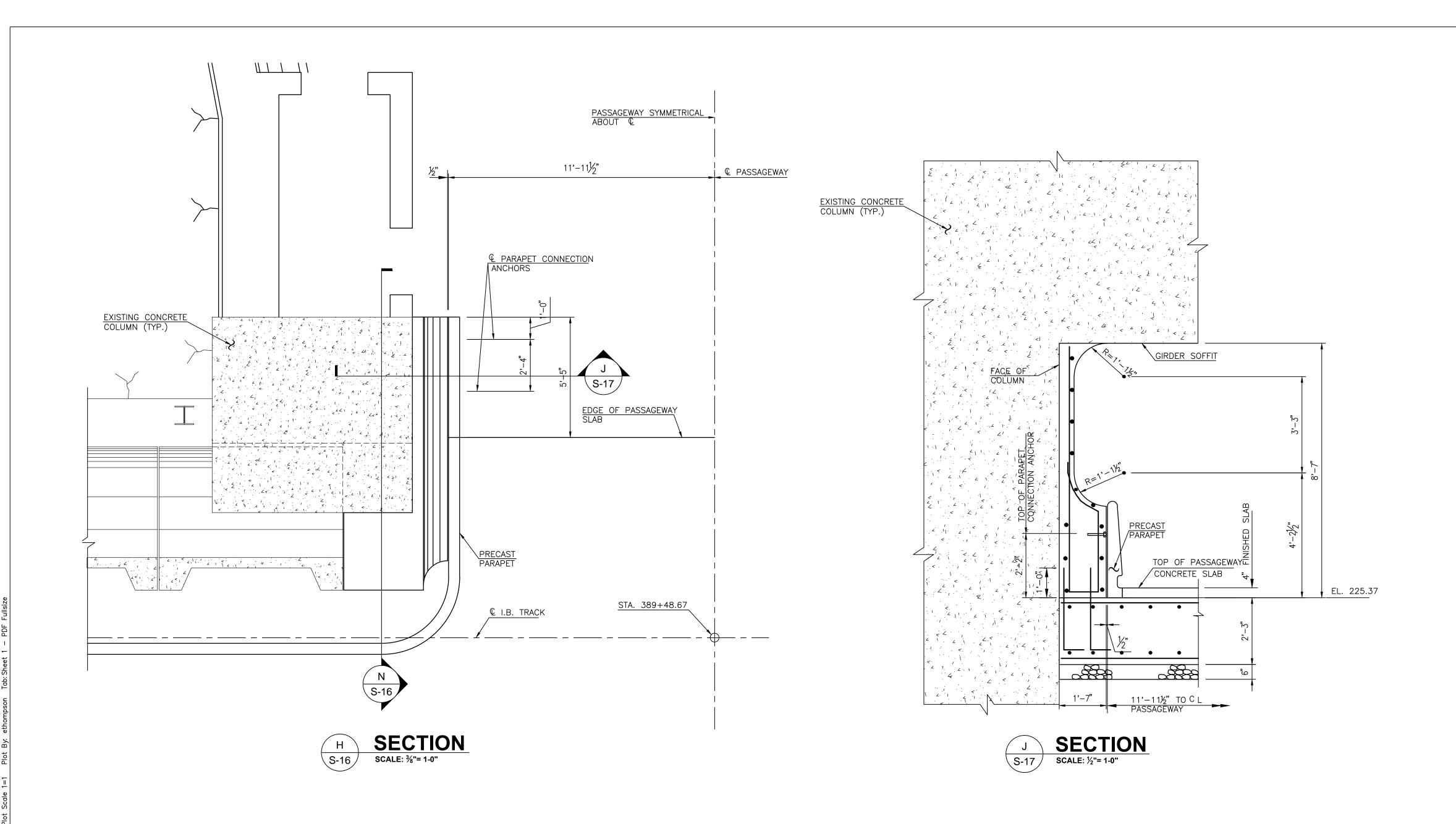
DATE BY

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OFFICE OF ENGINEERING SERVICE Rummel, Klepper & Kahl, LLP
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SUBMITTED BY_

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WHITMAN, REQUARDT & ASSOCIATES, LLP

BETHESDA STATION - SOUTH ENTRANCE

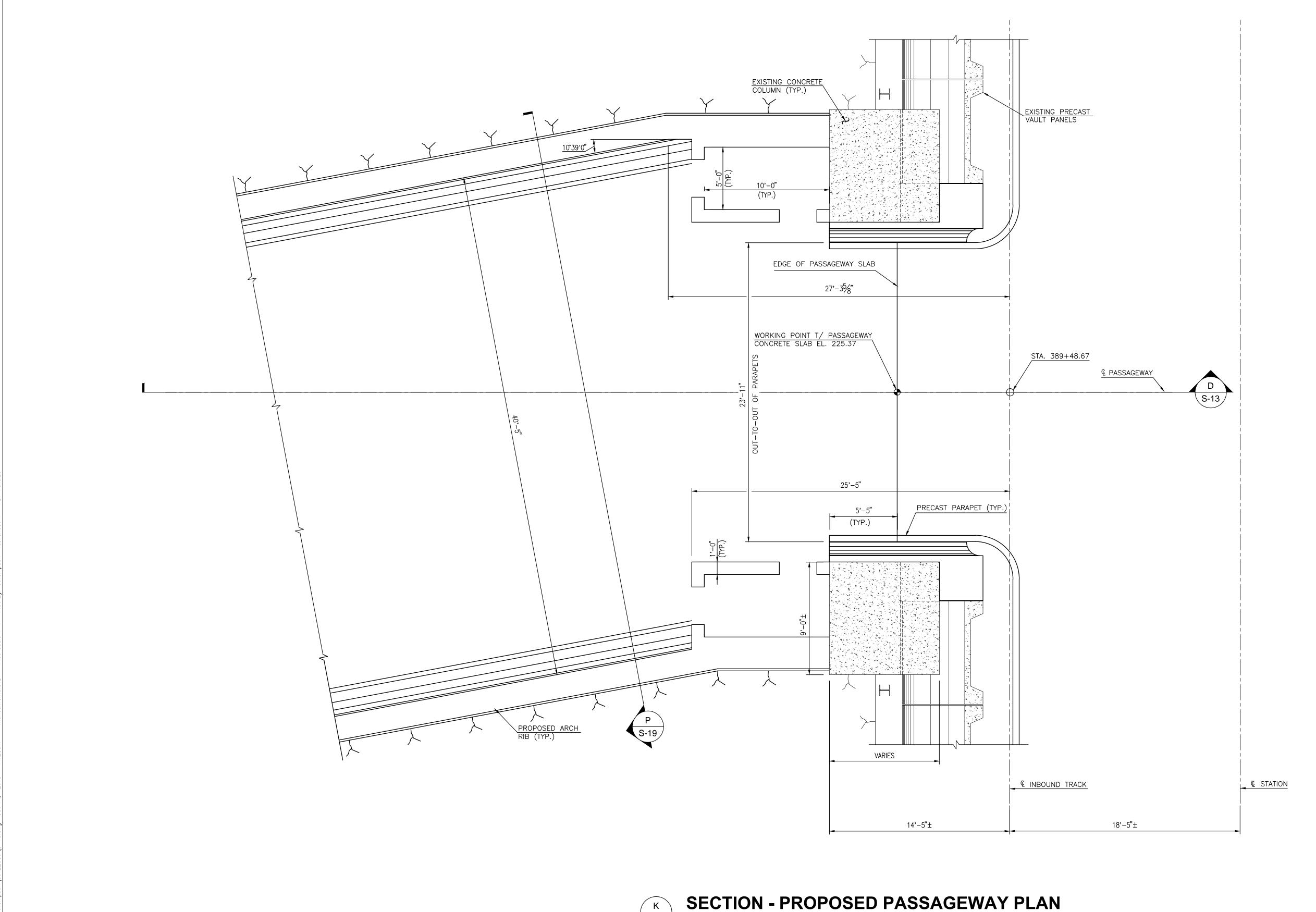
DETAILS AT SOUTH PASSAGEWAY

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S—17



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REVISIONS DESCRIPTION DRAWN E.M. THOMPSON DATE CHECKED D.S. TUSING



Maryland

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DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

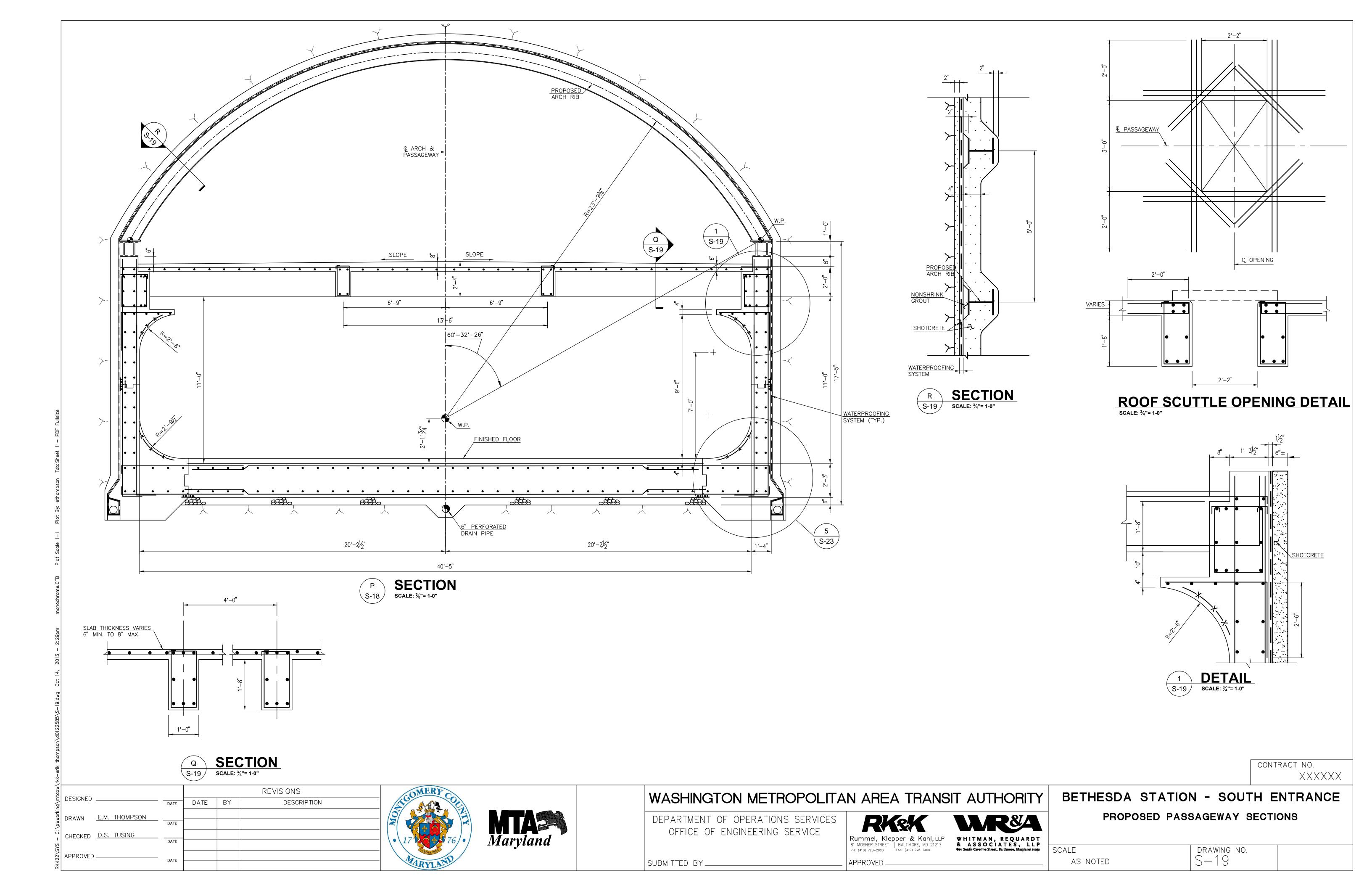
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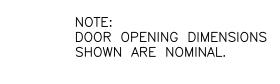
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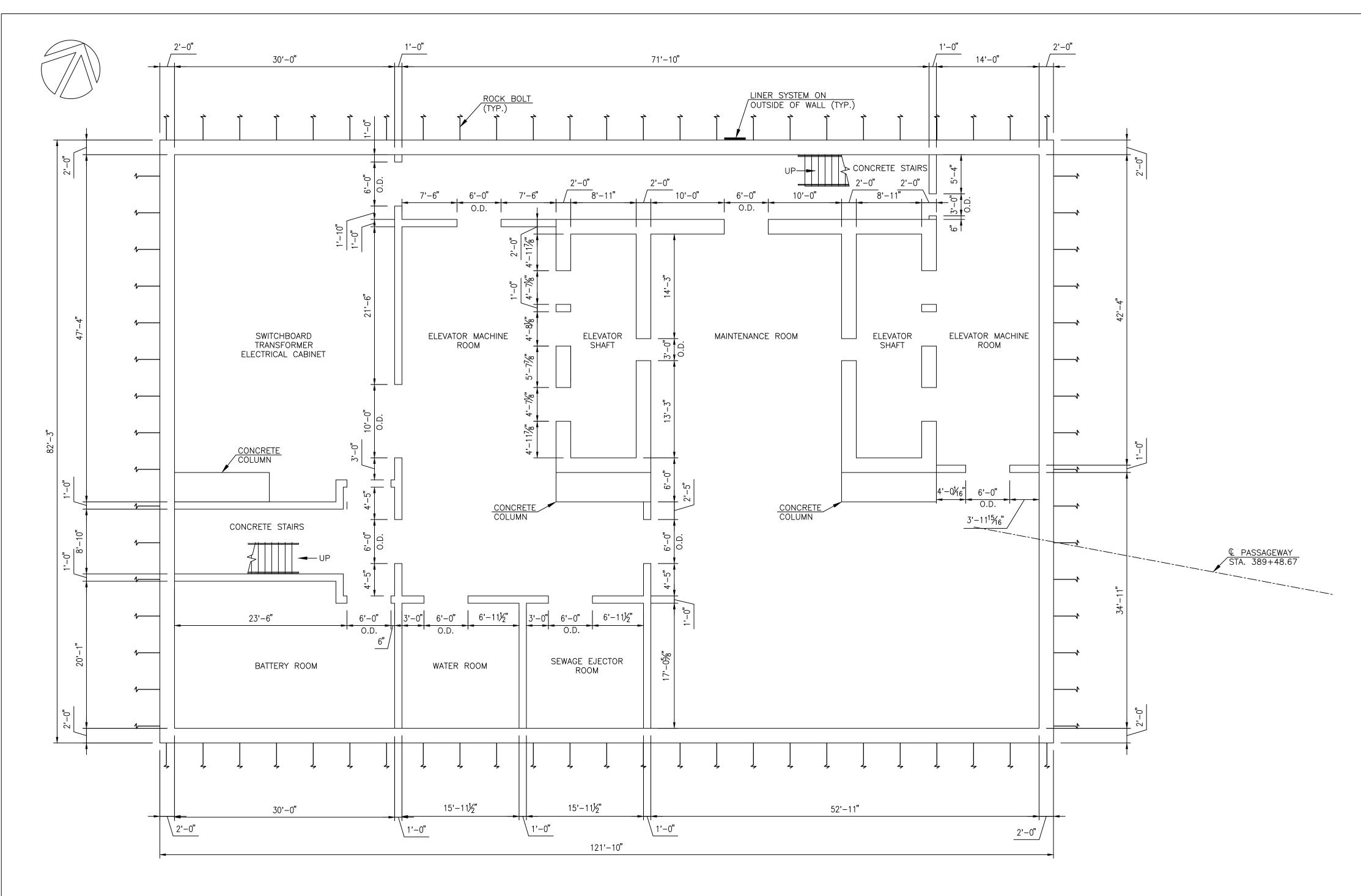
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BETHESDA	STATION	- SOUTH	ENTRANCE
PRO	POSED PAS	SAGEWAY F	PLAN

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ELEVATOR MACHINE ROOM FLOOR PLAN

SCALE: 1/8" = 1-0"

CONTRACT NO.

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WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

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SUBMITTED BY_

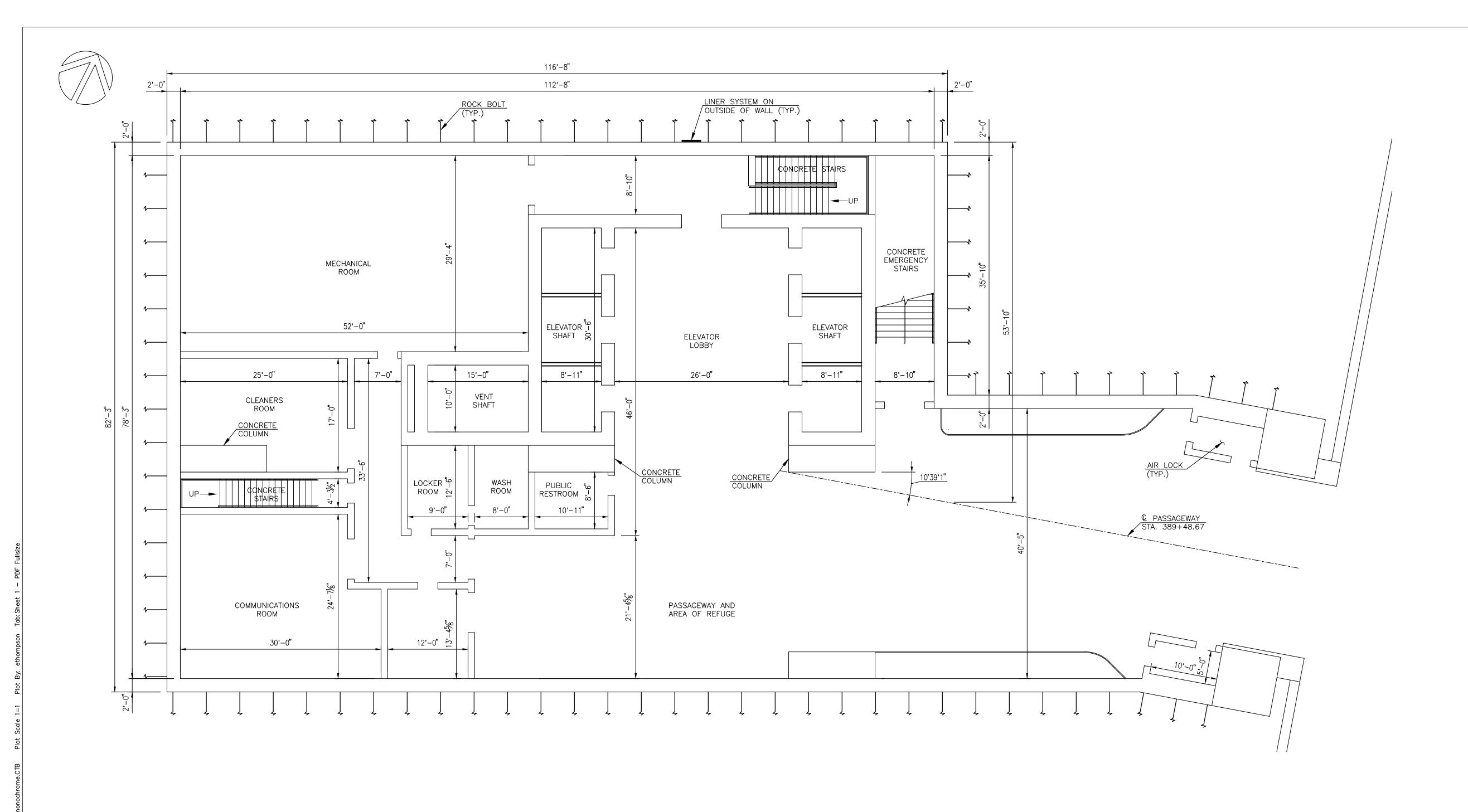
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BETHESDA STATION - SOUTH ENTRANCE

ELEVATOR MACHINE ROOM FLOOR PLAN

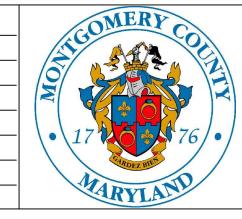
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MEZZANINE LEVEL FLOOR PLAN SCALE: $\frac{1}{8}$ = 1-0"

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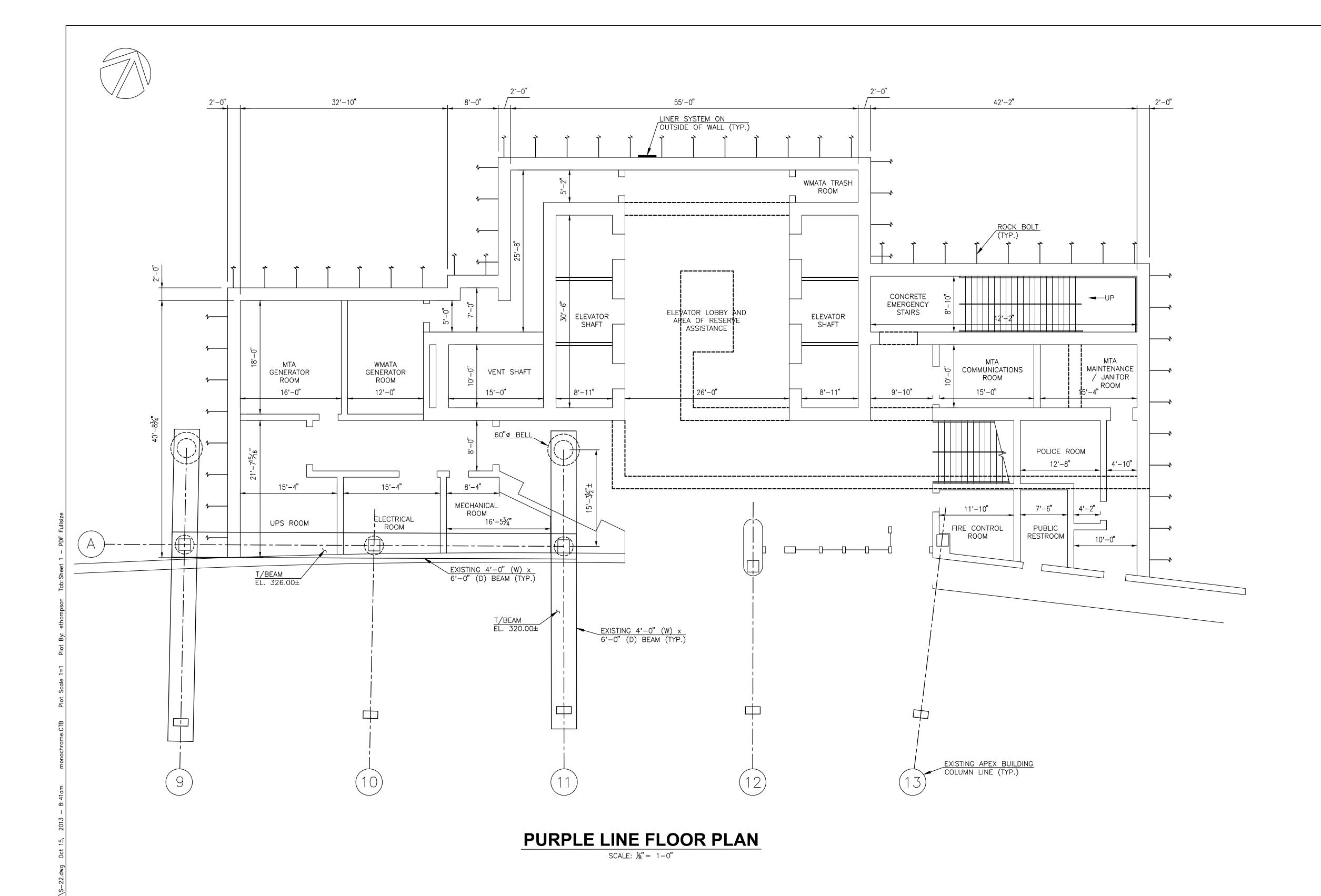
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& ASSOCIATES, LLP
Soil South Caroline Street, Baltimore, Maryland 21251

BETHESDA STATION - SOUTH ENTRANCE

MEZZANINE LEVEL FLOOR PLAN

DRAWING NO. S-21 SCALE AS NOTED



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DESIGNED DATE DATE BY DESCRIPTION

DRAWN E.M. THOMPSON DATE

CHECKED D.S. TUSING

APPROVED DATE





WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

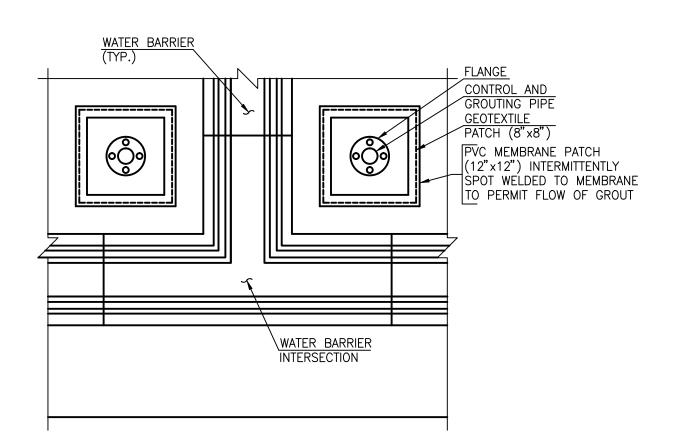
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PH: (410) 728-2900 FAX: (410) 728-3160

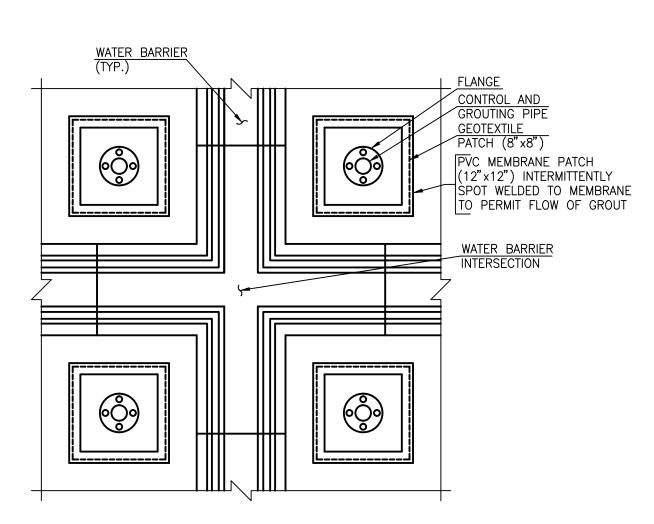
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BETHESDA STATION - SOUTH ENTRANCE PURPLE LINE FLOOR PLAN

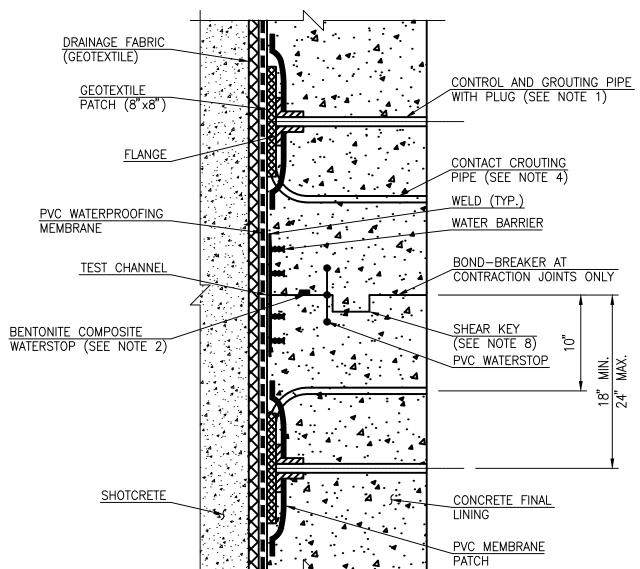
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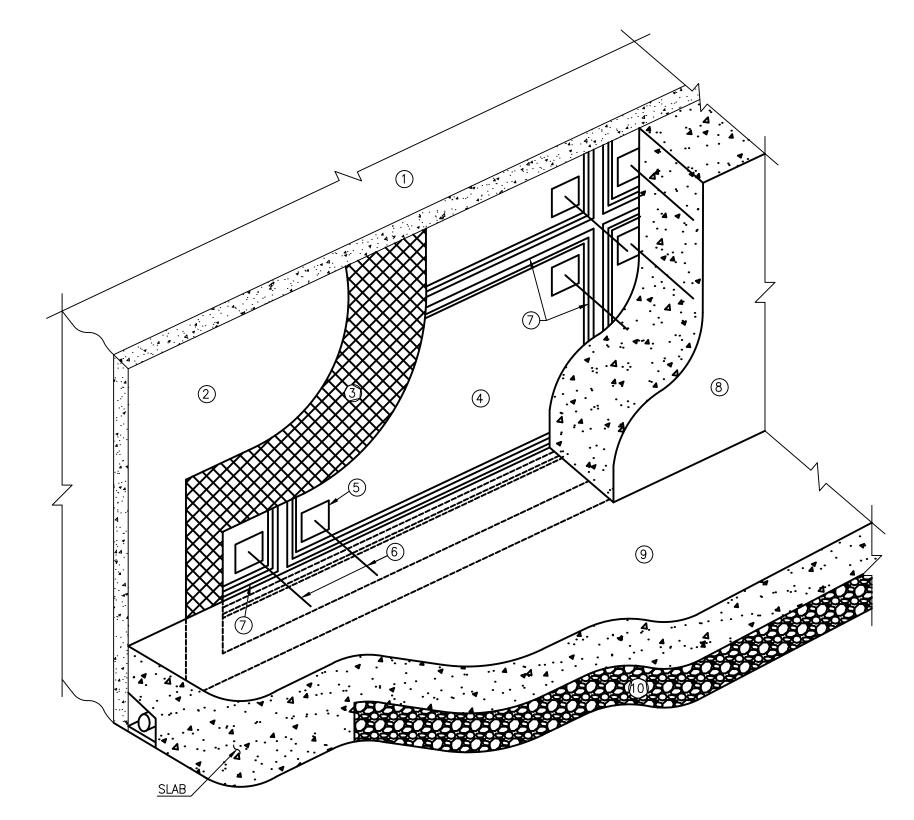






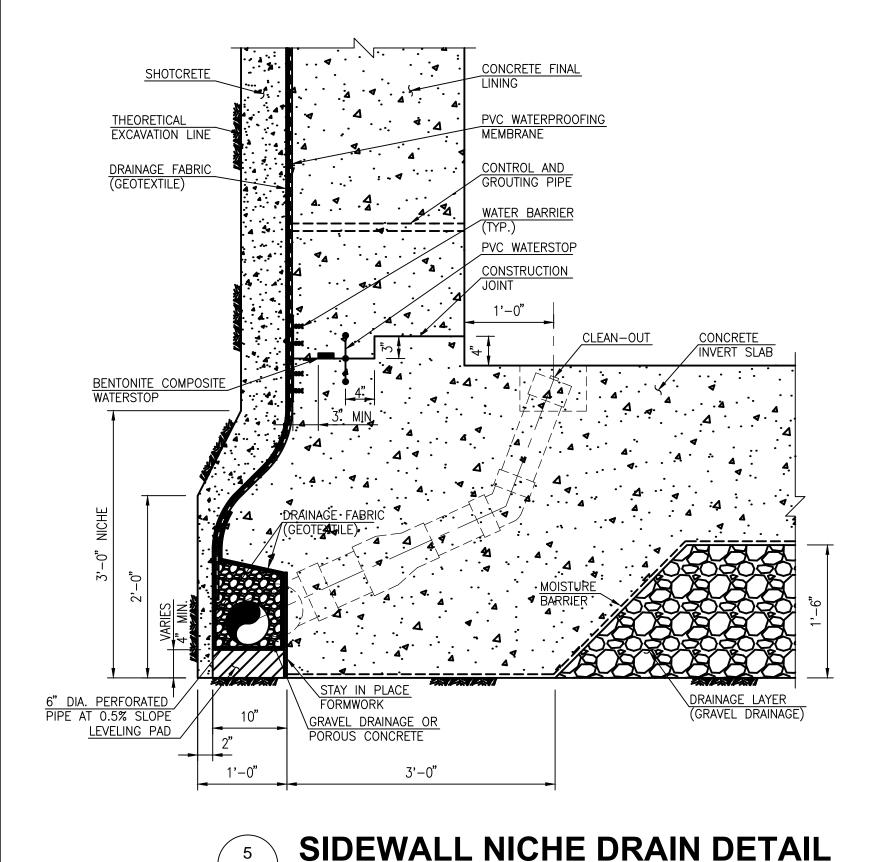


INSTEAD OF PVC WATERSTOP.

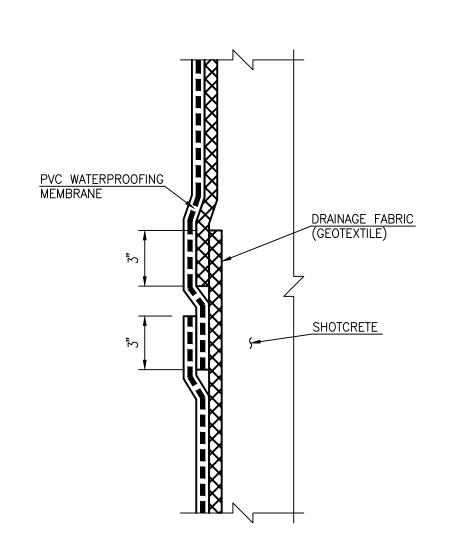




1. ROCK MASS SHOTCRETE DRAINAGE FABRIC (GEOTEXTILE) PVC WATERPROOFING MEMBRANE PVC MEMBRANE PATCH CONTROL AND GROUTING PIPE WATER BARRIER CONCRETE FINAL LINING CONCRETE INVERT SLAB 10. DRAINAGE LAYER



APPLICABLE TO: ELEVATOR AND STAIR SHAFT AND SINGLE LEVEL MEZZANINE SCALE: N.T.S.





OBSTRUCTIONS. 8. SEE STRUCTURAL DRAWINGS FOR SHEAR KEY AND REINFORCEMENT DETAILS. WELD CONNECTION BETWEEN MEMBRANE AND BA-ANCHOR WITH SINGLE WELD. CONTRACTOR SHALL DETERMINE OPTIMAL NUMBER AND LOCATION OF BA-ANCHORS. THE CONTRACTOR MAY PROPOSE ALTERNATIVE SYSTEM TO SUPPORT REINFORCING STEEL. CONTRACTOR SHALL SUBMIT ALTERNATIVE SYSTEM TO THE AR FOR REVIEW PRIOR TO SETTING

CONTROL AND GROUTING PIPES TO BE LOCATED CLEAR OF WALLS, FLOOR SLABS AND OTHER

1. SECURELY ATTACH CONTROL AND GROUTING PIPE TO

2. WHERE AN EXTERNAL WATER BARRIER IS INDICATED A

HORIZONTAL CONSTRUCTION JOINTS SHALL BE AS

INSTALL CONTACT GROUT PIERS ON A STAGGERED PATTERN, 12" FROM CENTER EACH SIDE OF

CENTERLINE, 5'-0" MAX. LONGITUDINAL SPACING.

INSTALL ADDITIONAL PIPES, AS REQUIRED. 5. LOCATIONS OF WATER BARRIERS ARE TO BE

SPACING OF WATER BARRIERS IS 35 FT. ENSURE CONTINUITY OF WATER BARRIERS.

PIPES SHALL HAVE THREADED COUPLINGS AND CAPS.

COORDINATED WITH CONSTRUCTION JOINTS. MAXIMUM

STEEL REINFORCEMENT. PIPE SHALL HAVE THREADED

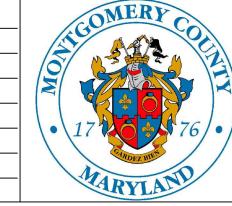
COUPLING FOR GROUT LINE AND REMOVABLE PLUG.

BENTONITE COMPOSITE WATERSTOP SHALL BE USED

REINFORCING STEEL.

CONTRACT NO. XXXXXX

REVISIONS DESIGNED _ DESCRIPTION DATE BY DATE DRAWN <u>E.M. THOMPSON</u> CHECKED D.S. TUSING APPROVED_





WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

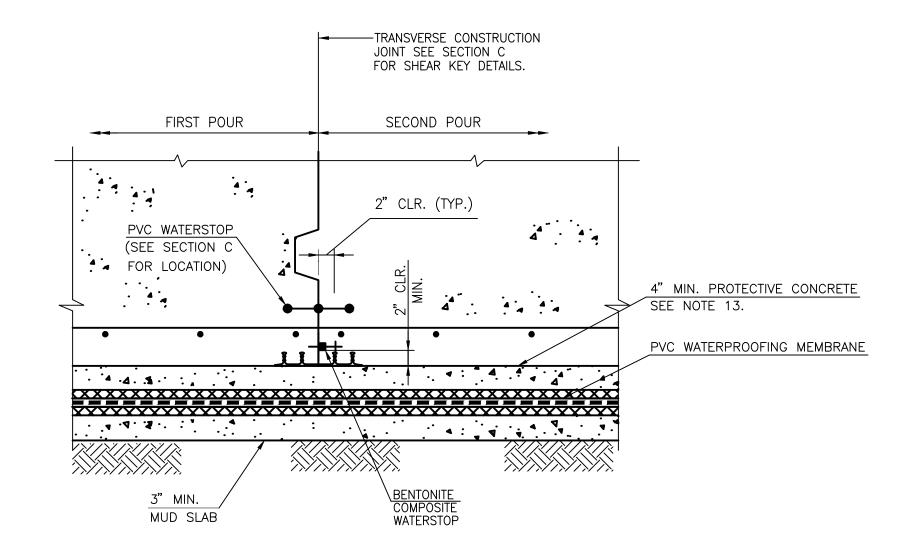
Rummel, Klepper & Kahl, LLP

WHITMAN, REQUARDT & ASSOCIATES, LLP

BETHESDA STATION - SOUTH ENTRANCE WATERPROOFING SYSTEM DETAILS - 1

SCALE DRAWING NO. AS NOTED

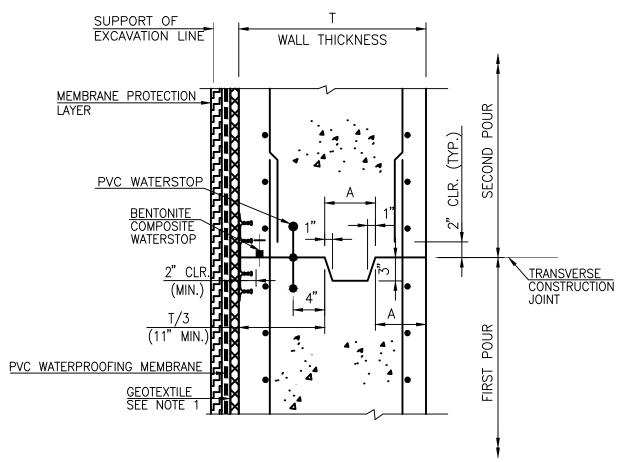
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SLAB SECTION

SCALE: N.T.S.

S-24 /



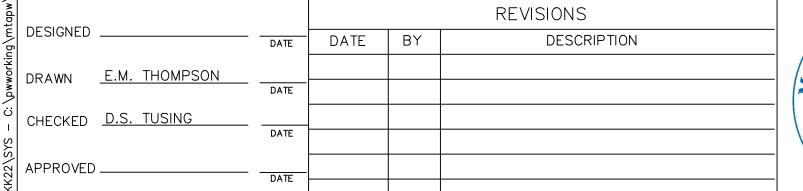




NOTES:

- 1. GEOTEXTILE TYPE 1 SHALL BE 22 OZ/SY (285 MIL). GEOTEXTILE TYPE 2 SHALL BE 28 OZ/SY (400 MIL). USE TYPE I AGAINST STRUCTURE WALL AND TYPE 2 AGAINST SUPPORT OF EXCAVATION. CHAMFER 2"x2" MIN. ALL CORNERS TO WHICH WATERPROOFING IS TO BE APPLIED.
- 2. MEMBRANE PROTECTION LAYER SHALL BE 60 MIL PVC MEMBRANE IN CONJUNCTION WITH A LAYER OF POLYSTYRENE OR 1/2" LAYER OF PLYWOOD.
- 3. PVC MEMBRANE SHALL BE NON REINFORCED 2.5MM (100 MIL) THICK. WELD PATCH USING CONTINUOUS HOT WELD SEAMS TO MEMBRANE ON EVERY END. PVC MEMBRANE PATCH TO BE INSTALLED ONLY ON CORNERS.
- 4. WATER BARRIER TO BE INSTALLED, U.O.N., AT 2' FROM TOP OF CONCRETE MUD SLAB. HANDWELD SPLICES BY SIDE WELD SEAMS.
- 5. PVC CONTROL AND GROUTING PIPE SHALL BE 1" NOMINAL DIAMETER, SCHEDULE 40.
- 6. CONTINUE VERTICAL WATER BARRIER ON THE ROOF AT PORTIONS OF STRUCTURE THAT ARE FULLY UNDERGROUND.
- 7. WATERPROOFING DETAILS FOR ALL CUT AND COVER STRUCTURES TO BE CONTINUOUS ALL AROUND FOR THE ENTIRE LENGTH OF THE STRUCTURES.
- 8. BENTONITE COMPOSITE WATERSTOP, AND PVC WATERSTOP (DUMBBELL TYPE, CENTER BULB, 9 INCH WIDTH, 3/8 INCH STEM THICKNESS, 3/4 INCH BULBS) ARE TYPICAL FOR EXTERIOR TRANSVERSE CONSTRUCTION JOINTS IN ROOFS, WALLS AND INVERT SLABS.
- 9. THE MINIMUM SIZE OF BENTONITE COMPOSITE WATERSTOP SHALL BE 3/4 INCH THICK AND 1 INCH WIDE.
- 10. BENTONITE COMPOSITE WATERSTOP TO BE PLACED BETWEEN PVC WATERSTOP AND EXTERIOR FACE OF WALL OR SLAB, AS SHOWN. PROVIDE A MINIMUM OF 2" OF CONCRETE COVER ADJACENT TO THE WATERSTOP.
- 11. BENTONITE COMPOSITE WATERSTOP IN HORIZONTAL LONGITUDINAL DIRECTION SHALL BE LINED UP & ADHERED TO BENTONITE IN VERTICAL DIRECTION AT TRANSVERSE CONSTRUCTION JOINTS.
- 12. PVC WATERSTOP IN HORIZONTAL LONGITUDINAL DIRECTION SHALL BE LINED UP & WELDED TO VERTICAL PVC WATERSTOP AT TRANSVERSE CONSTRUCTION JOINTS.
- 13. PROTECTIVE CONCRETE LAYER FOR ROOF SLAB OR INVERT WATERPROOFING SHALL BE REINFORCED WITH WELDED WIRE FABRIC IF TRAFFIC AND/OR EQUIPMENT IS EXPECTED TO RUN ON TOP.

CONTRACT NO. XXXXXX







WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

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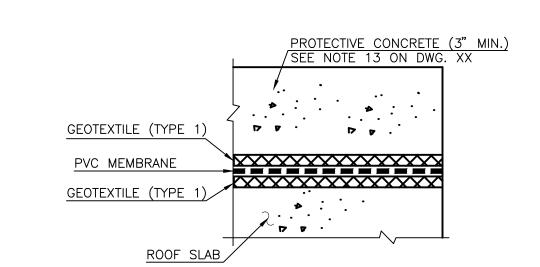
SUBMITTED BY_

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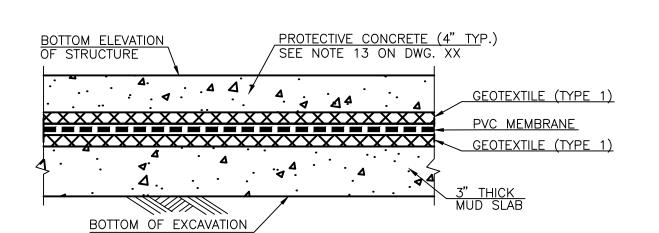


BETHESDA STATION - SOUTH ENTRANCE WATERPROOFING SYSTEM DETAILS - 2

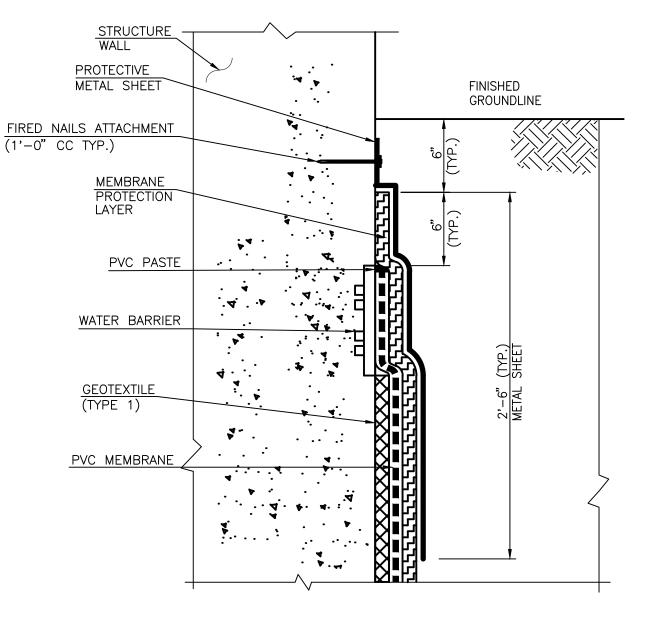
SCALE DRAWING NO. S - 24AS NOTED



WATERPROOFING AT ROOF SLAB S-25 / SCALE: N.T.S.



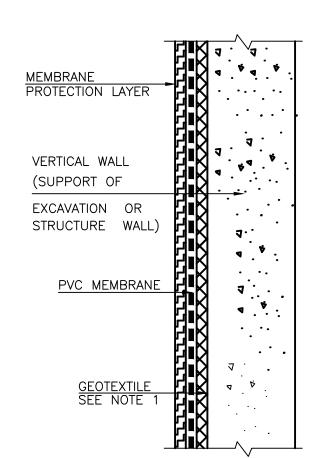
INVERT WATERPROOFING DETAIL S-25 SCALE: N.T.S.



AT STRUCTURAL WALL

FINISHED GROUNDLINE : 44 STRUCTURE WALL EXCAVATION AT SUPPORT OF EXCAVATION WALL

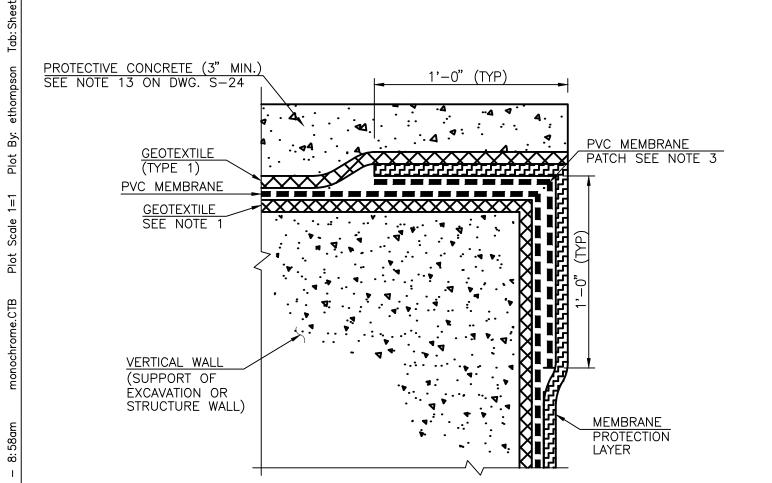
WATERPROOFING TERMINATION **NEAR GROUND SURFACE DETAIL**



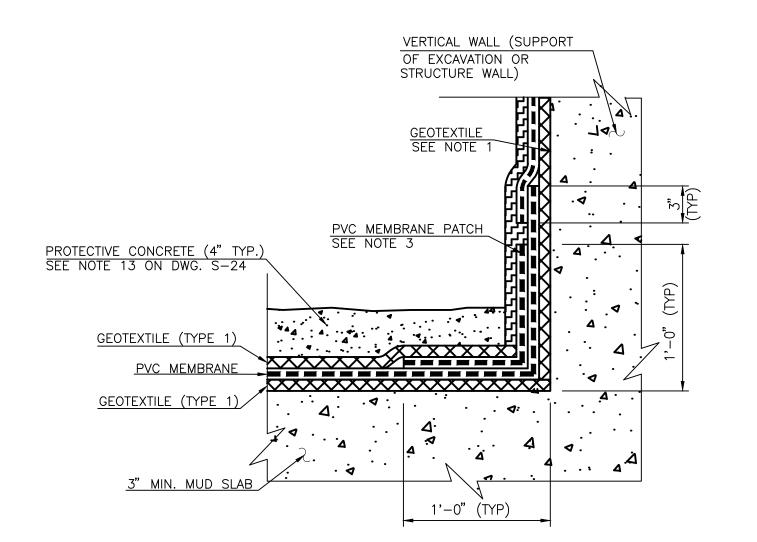
NOTES:

- 1. GEOTEXTILE TYPE 1 SHALL BE 22 OZ/SY (285 MIL) GEOTEXTILE TYPE 2 SHALL BE 28 OZ/SY (400 MIL) USE TYPE I AGAINST STRUCTURE WALL AND TYPE 2 AGAINST SUPPORT OF EXCAVATION. CHAMFER ALL CORNERS TO WHICH WATERPROOFING
- 2. MEMBRANE PROTECTION LAYER SHALL BE 60 MIL PVC MEMBRANE IN CONJUNCTION WITH A LAYER OF POLYSTYRENE OR 1/2" LAYER OF PLYWOOD.
- 3. PVC MEMBRANE SHALL BE NON REINFORCED 2.5MM (100 MIL) THICK. WELD PATCH USING CONTINUOUS HOT WELD SEAMS TO MEMBRANE ON EVERY END. PVC MEMBRANE PATCH TO BE INSTALLED ONLY ON CORNERS.
- 4. REVIEW THIS SHEET ALONG WITH DWGS. NOO-NGA-S-503, NOO-NGA-S-505, AND N00-NGA-S-506.
- 5. ALL CONCRETE CORNERS SHALL BE CHAMFERED 2 IN. x 2 IN.
- 6. FOR ADDITIONAL NOTES, SEE DWG. S-24.

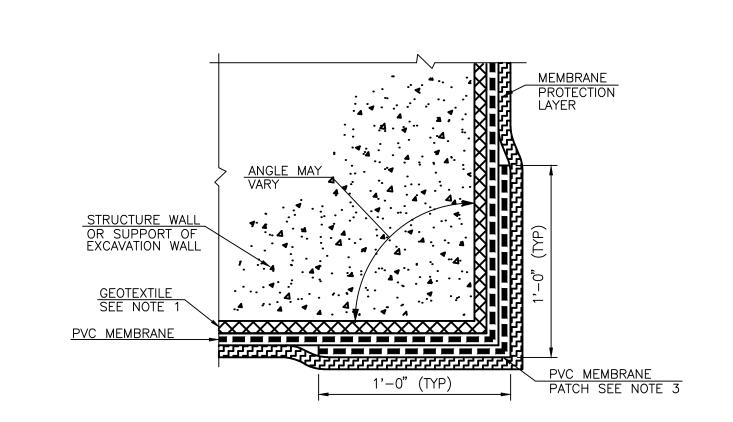




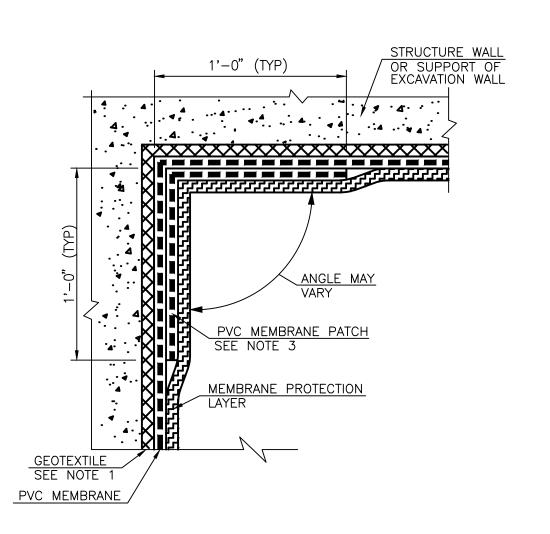
WATERPROOFING SLAB/WALL TRANSITION DETAIL AT **OUTSIDE CORNERS**



WATERPROOFING SLAB/WALL TRANSITION DETAIL AT **INSIDE CORNERS** SCALE: N.T.S.



WATERPROOFING TRANSITION DETAIL AT VERTICAL OUTSIDE CORNERS



WATERPROOFING TRANSITION DETAIL AT VERTICAL INSIDE CORNERS

> CONTRACT NO. XXXXXX

REVISIONS DESIGNED DESCRIPTION DATE DATE DRAWN E.M. THOMPSON CHECKED D.S. TUSING APPROVED -





WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

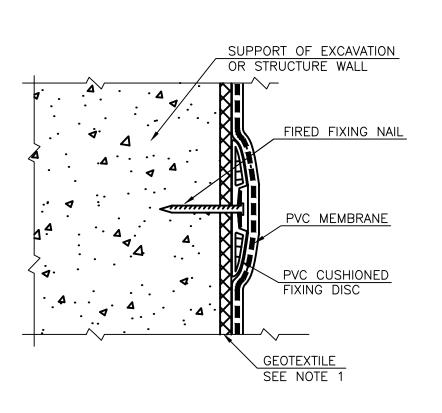
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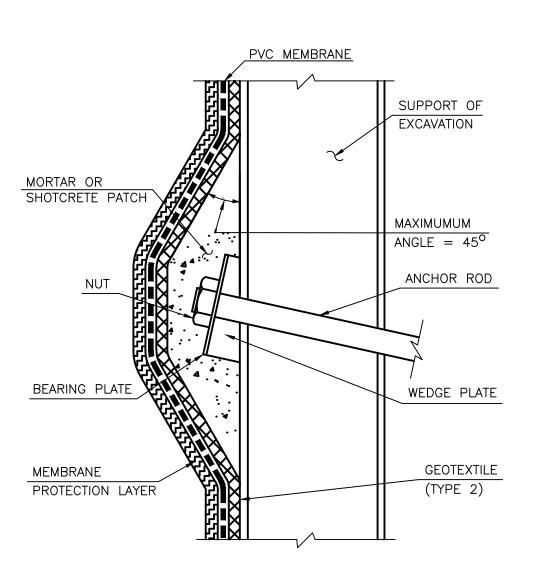
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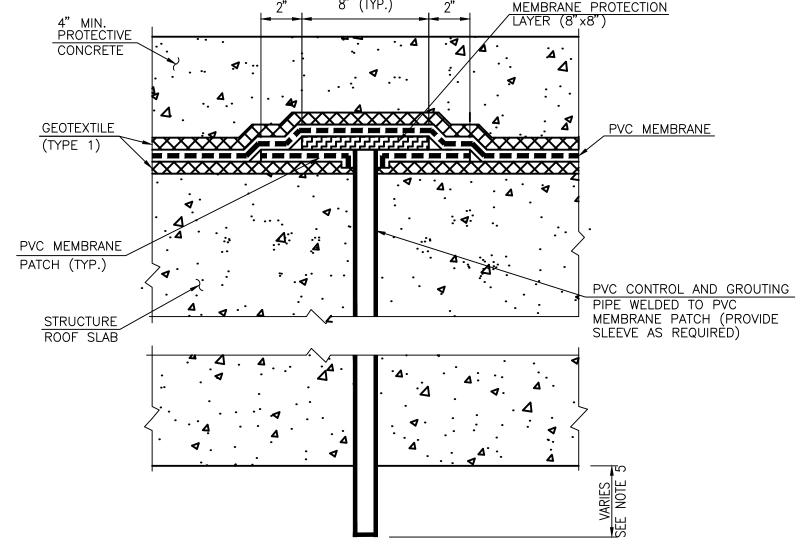
WHITMAN, REQUARDT & ASSOCIATES, LLP

BETHESDA STATION - SOUTH ENTRANCE WATERPROOFING SYSTEM DETAILS - 3

DRAWING NO. S - 25AS NOTED



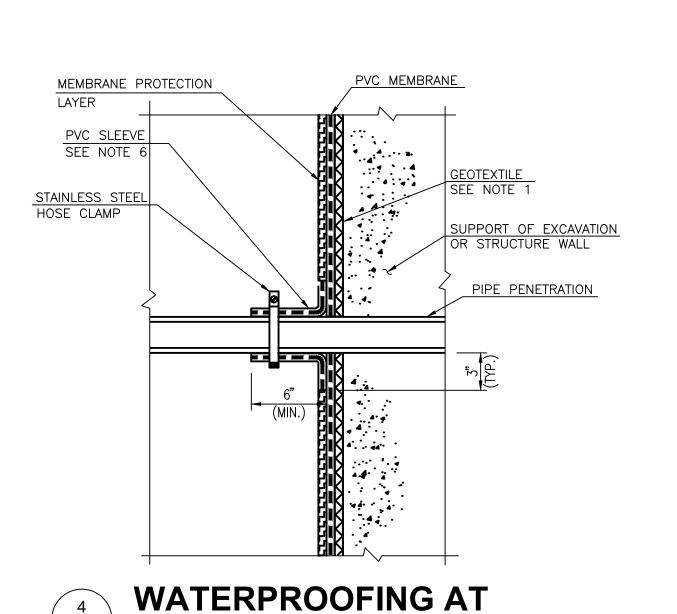


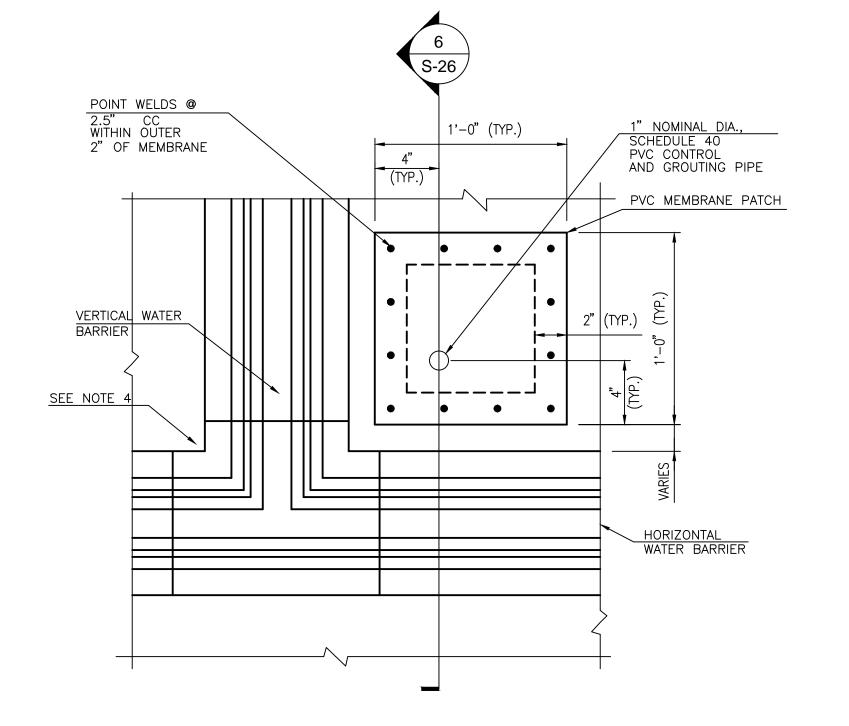


CONTROL AND GROUTING PIPE IN ROOF SLAB SCALE: N.T.S.

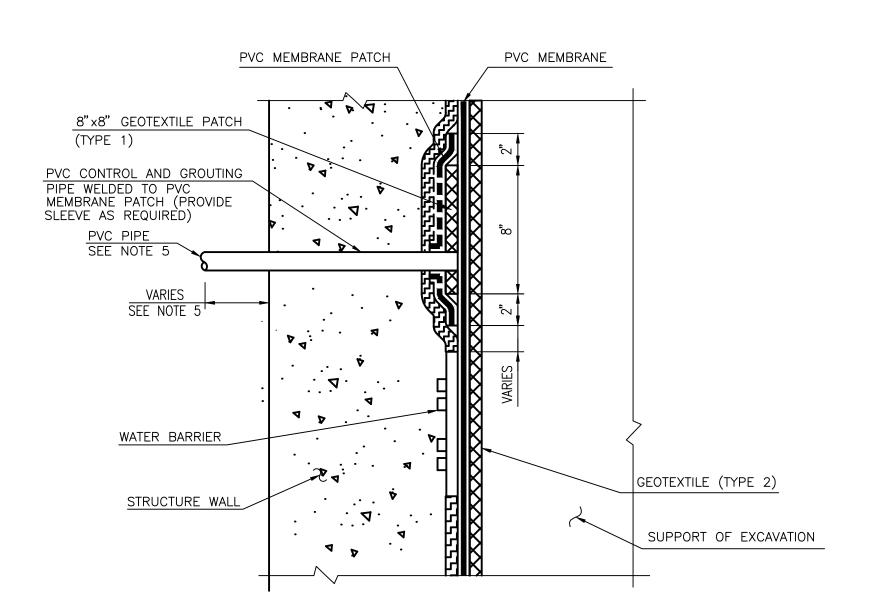












NOTES:

1. GEOTEXTILE TYPE 1 SHALL BE 22 OZ/SY (285 MIL) GEOTEXTILE TYPE 2 SHALL BE 28 OZ/SY (400 MIL).

CHAMFER ALL CORNERS TO WHICH WATERPROOFING

2. MEMBRANE PROTECTION LAYER SHALL BE 60 MIL PVC

MEMBRANE IN CONJUNCTION WITH A LAYER OF

3. PVC MEMBRANE SHALL BE NON REINFORCED 2.5MM

5. WHEN VISIBLE IN PUBLIC AREAS, PROVIDE PVC PIPE

WITH INSIDE THREADS AND REMOVABLE END CAP

FLUSH WITH THE CONCRETE. WHEN NOT VISIBLE IN

PUBLIC AREAS, PROVIDE 4" EXTENSION WITH OPEN END

(100 MIL) THICK. WELD PATCH USING CONTINUOUS HOT WELD SEAMS TO MEMBRANE ON EVERY END.

4. WATER BARRIER INTERSECTION TO BE PREMANUFACTURED

BY MANUFACTURER OR AT WORKSHOP ON SITE. HANDWELD

SPLICES BY SIDE WELD SEAMS. OTHER INTERSECTION TYPES

POLYSTYRENE OR 1/2" LAYER OF PLYWOOD.

USE SIMILAR CONNECTION.

BEYOND CONCRETE SURFACE.

PVC PIPE AND SLEEVE.

6. APPLY SILICONE PASTE OR EQUAL BETWEEN

7. FOR ADDITIONAL NOTES, SEE DWG. S-24.

USE TYPE I AGAINST STRUCTURE WALL AND TYPE 2 AGAINST SUPPORT OF EXCAVATION.



CONTRACT NO. XXXXXX

REVISIONS DESCRIPTION DATE BY DATE DRAWN <u>E.M. THOMPSON</u> CHECKED D.S. TUSING

PIPE PENETRATIONS

SCALE: N.T.S.





WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES OFFICE OF ENGINEERING SERVICE

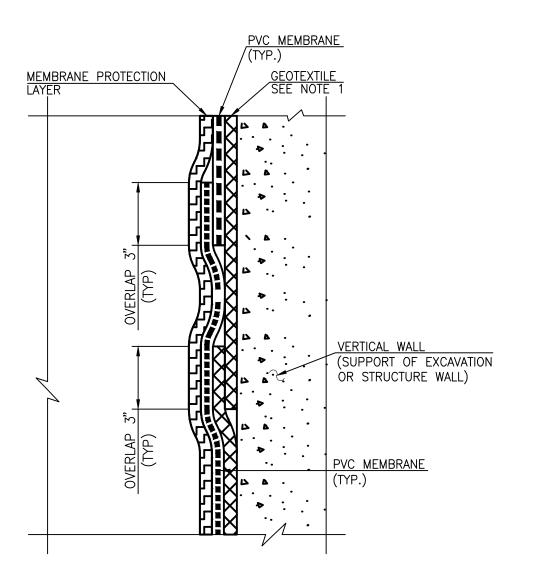
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WR84 WHITMAN, REQUARDT & ASSOCIATES, LLP BETHESDA STATION - SOUTH ENTRANCE WATERPROOFING SYSTEM DETAILS - 4

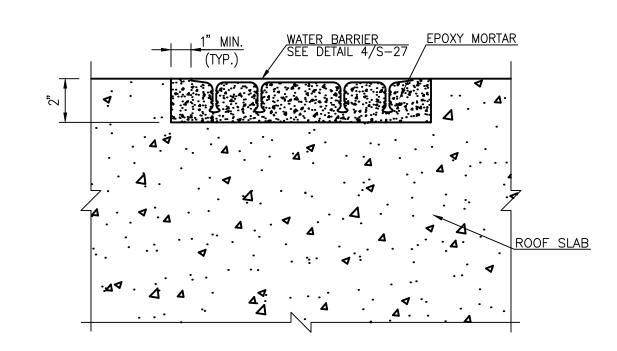
DRAWING NO. S - 26AS NOTED

SUBMITTED BY.

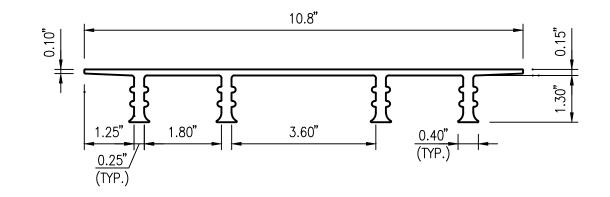
WATERPROOFING CONNECTION FROM SUPPORT
S-27 OF EXCAVATION WALL TO STRUCTURE SLAB
SCALE: N.T.S.



WATERPROOFING CONNECTION
SCALE: N.T.S.



WATER BARRIER INSTALLATION S-27 IN ROOF SLAB SCALE: N.T.S.





NOTES:

- 1. GEOTEXTILE TYPE 1 SHALL BE 22 OZ/SY (285 MIL).
 GEOTEXTILE TYPE 2 SHALL BE 28 OZ/SY (400 MIL).
 USE TYPE I AGAINST STRUCTURE WALL AND
 TYPE 2 AGAINST SUPPORT OF EXCAVATION.
 CHAMFER ALL CORNERS TO WHICH WATERPROOFING
 IS TO BE APPLIED.
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- 3. PVC MEMBRANE SHALL BE NON REINFORCED 2.5MM (100 MIL) THICK. WELD PATCH USING CONTINUOUS HOT WELD SEAMS TO MEMBRANE ON EVERY END. PVC MEMBRANE PATCH TO BE INSTALLED AS SHOWN.
- 4. WATER BARRIER SPLICES TO BE HANDWELDED BY SIDE WELD SEAMS.
- 5. FOR ADDITIONAL NOTES, SEE DWG. S-24.

CONTRACT NO.

XXXXXX

DESIGNED DATE DATE DATE DESCRIPTION

DRAWN E.M. THOMPSON DATE

CHECKED D.S. TUSING

APPROVED DATE





WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

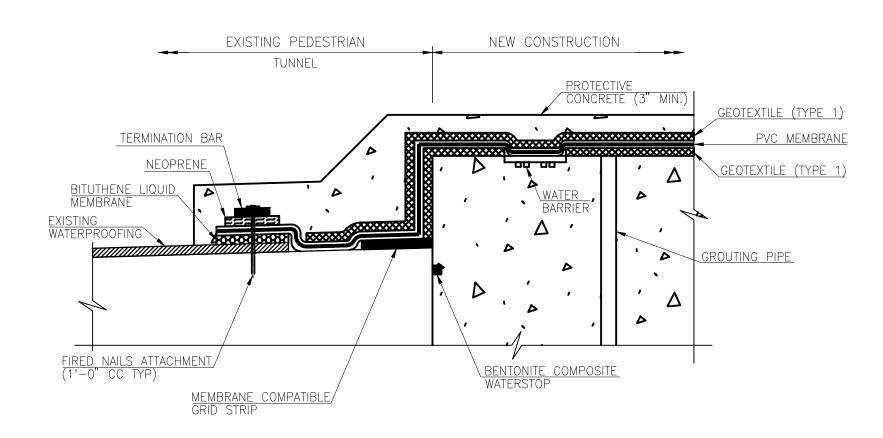
SUBMITTED BY_

Rummel, Klepper & Kahl, LLP
81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728-2900 FAX: (410) 728-3160



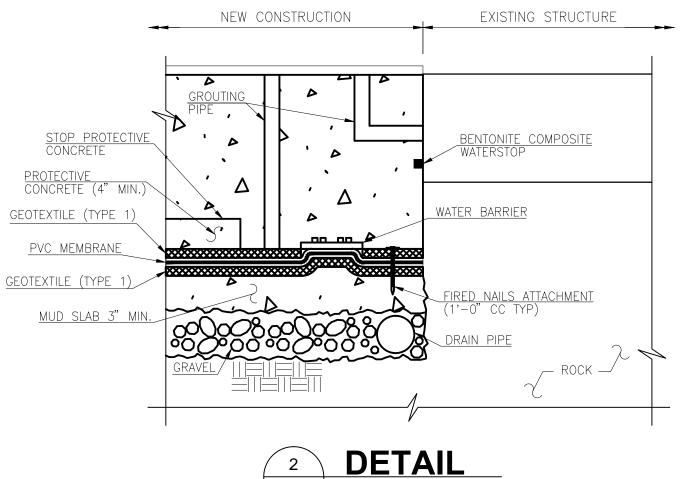
BETHESDA STATION - SOUTH ENTRANCE
WATERPROOFING SYSTEM DETAILS - 5

SCALE DRAWING NO. S-27



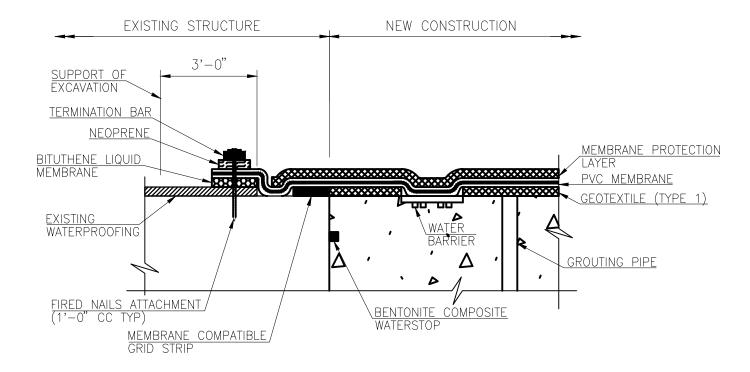
DETAIL

SCALE: N.T.S.



∖ S-28 */*

SCALE: N.T.S.





NOTES:

- 1. CONTRACTOR SHALL REMOVE EXISTING WATERPROOFING
 AT THE INTERFACE OF EXISTING STRUCTURES WITH NEW
 CONSTRUCTION TO ALLOW FOR ATTACHMENT AND TERMINATION
 OF NEW WATERPROOFING AS SHOWN IN THE DETAILS.
- 2. EXISTING CONCRETE SURFACES TO WHICH NEW WATERPROOFING IS TO BE ADHERED SHALL BE CLEANED AND PREPARED AS PER MANUFACTURER'S RECOMMENDATIONS.
- 3. CONTRACTOR SHALL PREVENT CONTACT BETWEEN THE PVC MEMBRANE AND ANY BITUMINOUS MATERIAL, INCLUDING EXISTING STRUCTURE WATERPROOFING.
- 4. MATERIALS FOR TERMINATION OF PVC WATERPROOFING AT EXISTING STRUCTURES, SUCH AS EPOXY ADHESIVE, BITUTHENE LIQUID MEMBRANE, NEOPRENE AND TERMINATION BAR, SHALL BE ACCORDING TO PVC MEMBRANE MANUFACTURER'S SPECIFICATIONS.
- 5. PROTECTIVE CONCRETE TO BE REINFORCED WITH A WELDED WIRE FABRIC IF EQUIPMENT IS EXPECTED TO RUN ON TOP.
- 6. FOR CORNER WATERPROOFING DETAIL, SEE DETAIL ON DWG. S-25.

CONTRACT NO.

tapw					REVISIONS	
ng\mt	DESIGNED	DATE	DATE	BY	DESCRIPTION	
pwworkin	DRAWN <u>E.M. THOMPSON</u>	DATE				
S - C:	CHECKED <u>D.S. TUSING</u>	DATE				
(K22\SY	APPROVED	DATE				}







DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

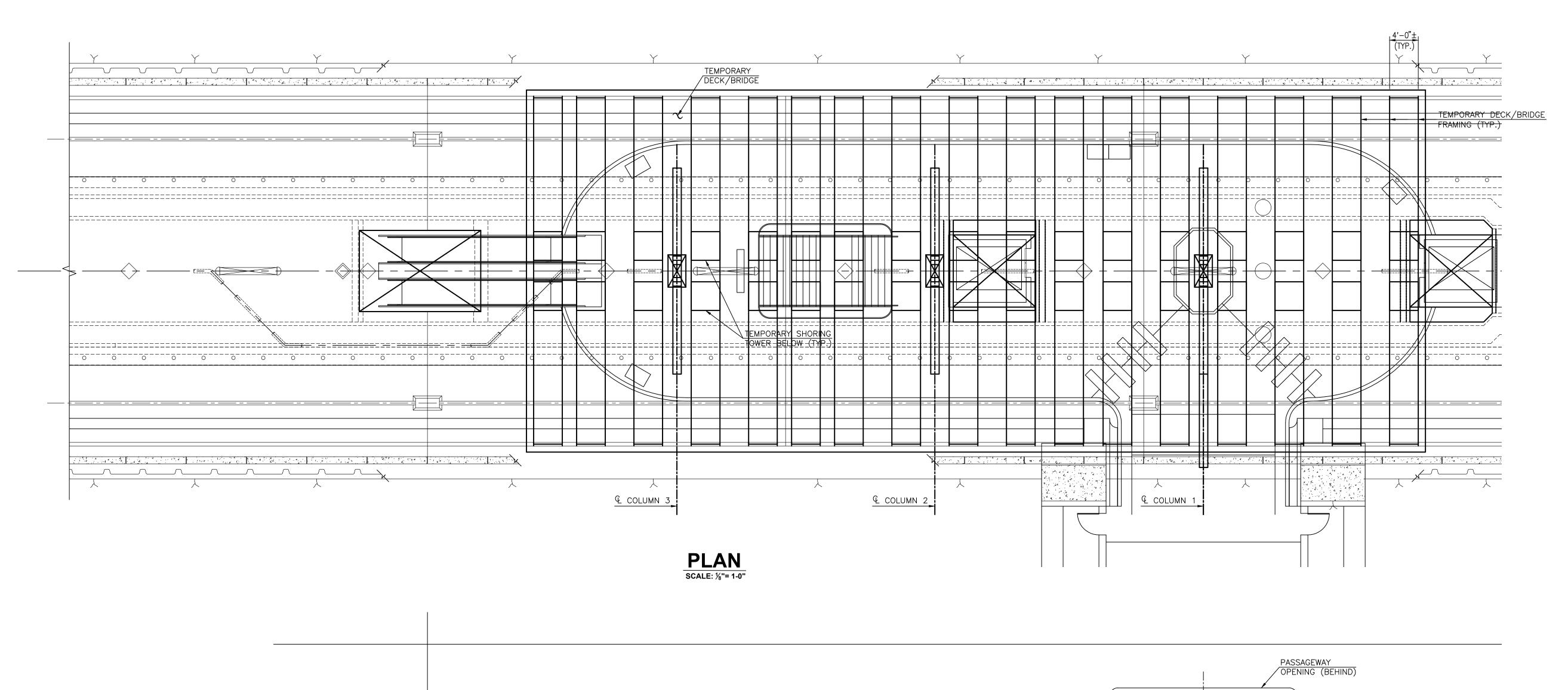
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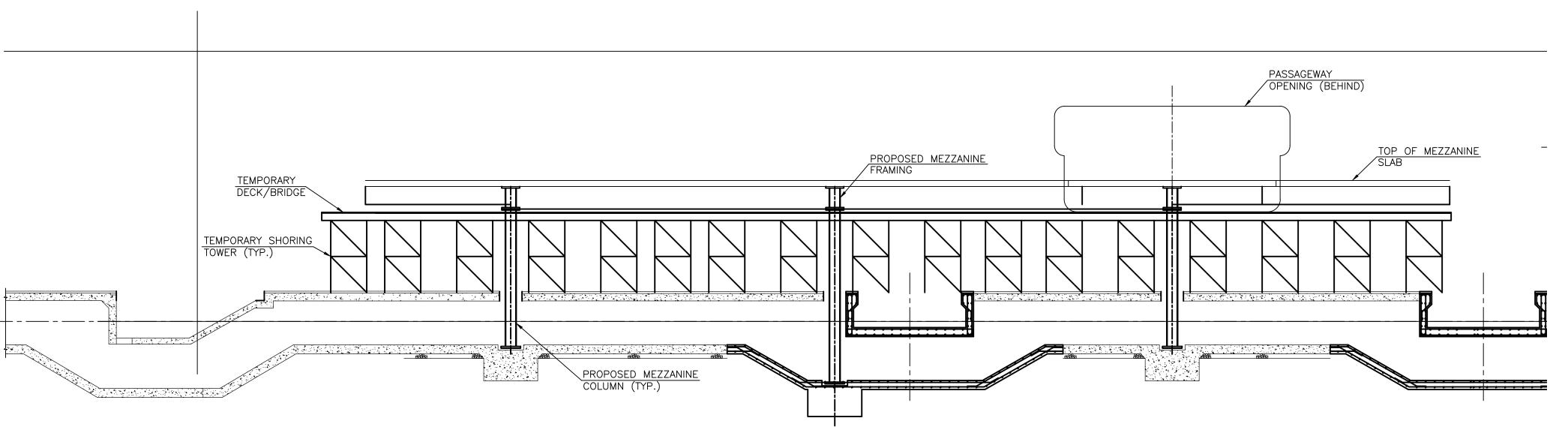
Rummel, Klepper & Kahl, LLP
81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728-2900 FAX: (410) 728-3160



BETHESDA STATION - SOUTH ENTRANCE WATERPROOFING SYSTEM DETAILS - 6

CALE	DRAWING NO.
AS NOTED	S-28





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SCALE: 1/8"= 1-0"

CONTRACT NO.

DESIGNED DATE DATE BY DESCRIPTION

DRAWN E.M. THOMPSON DATE

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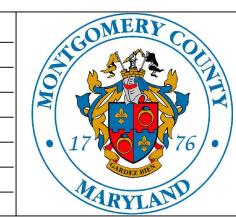
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WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF OPERATIONS SERVICES
OFFICE OF ENGINEERING SERVICE

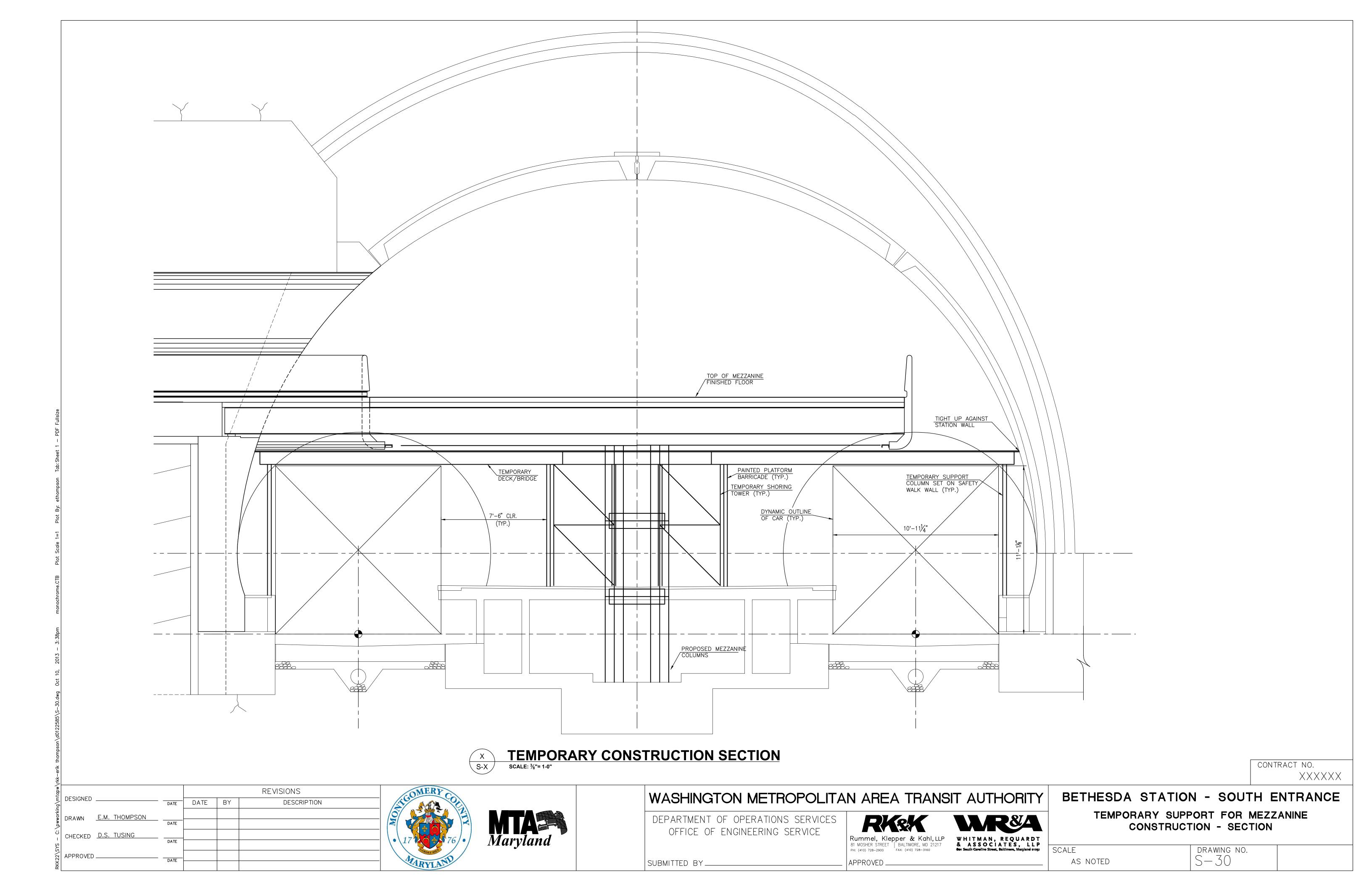
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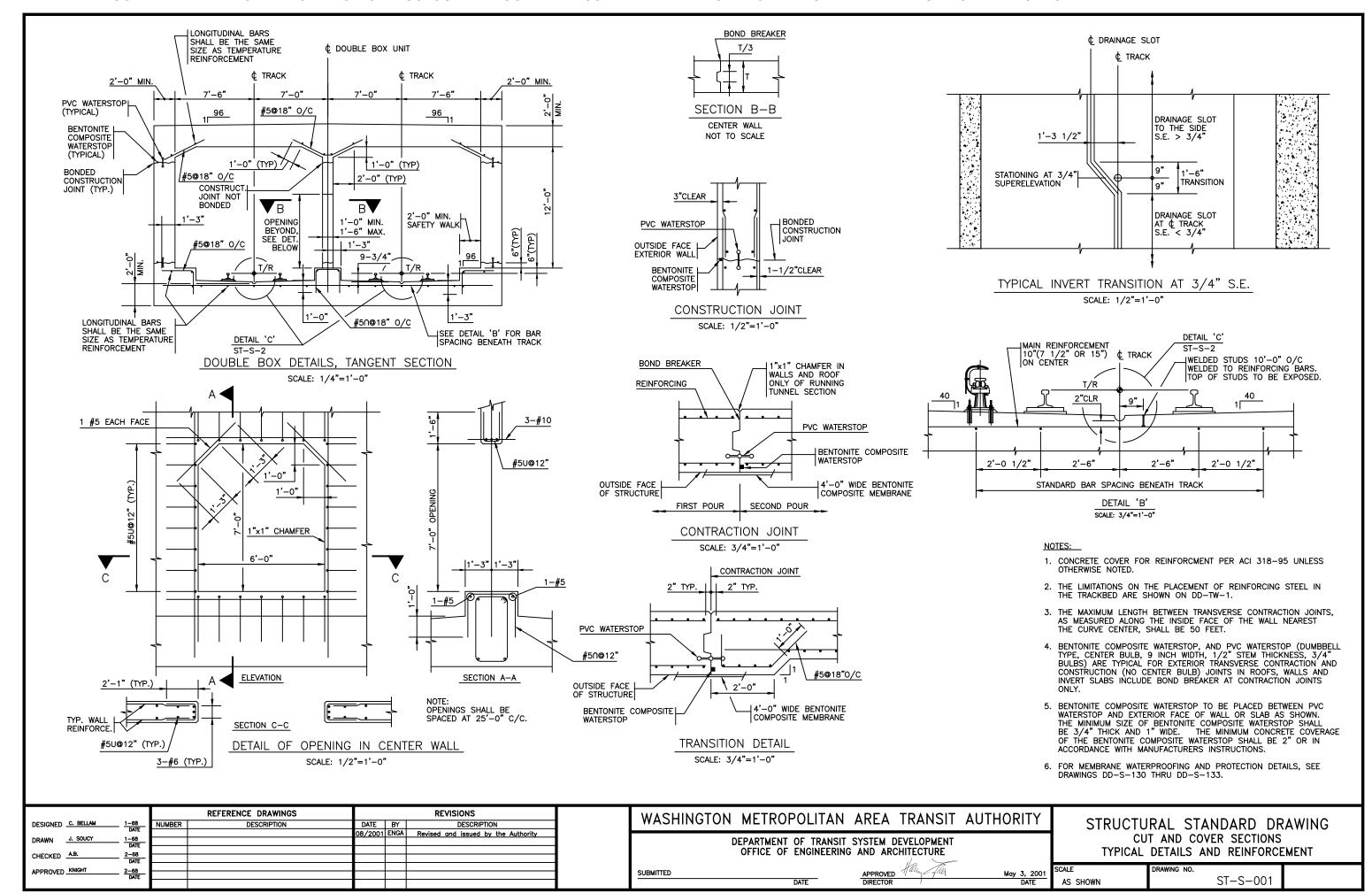
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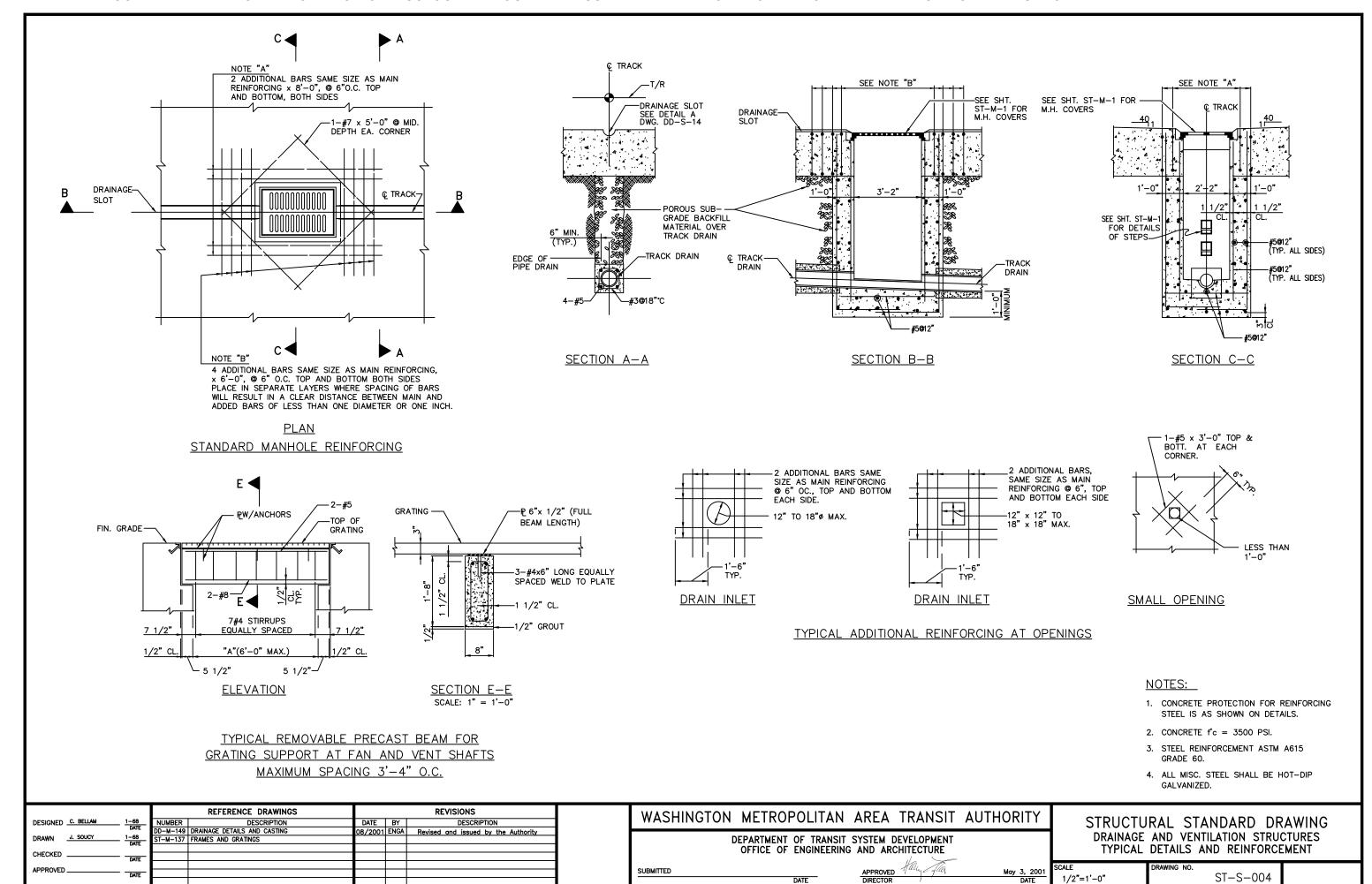
WHITMAN, REQUARDT
A ASSOCIATES, LLP
Ser South Caroline Street, Baltimore, Maryland 21291

BETHESDA STATION - SOUTH ENTRANCE BETHESDA STATION TEMPORARY SUPPORT FOR MEZZANINE CONSTRUCTION STA. 391+07 TO STA. 389+07

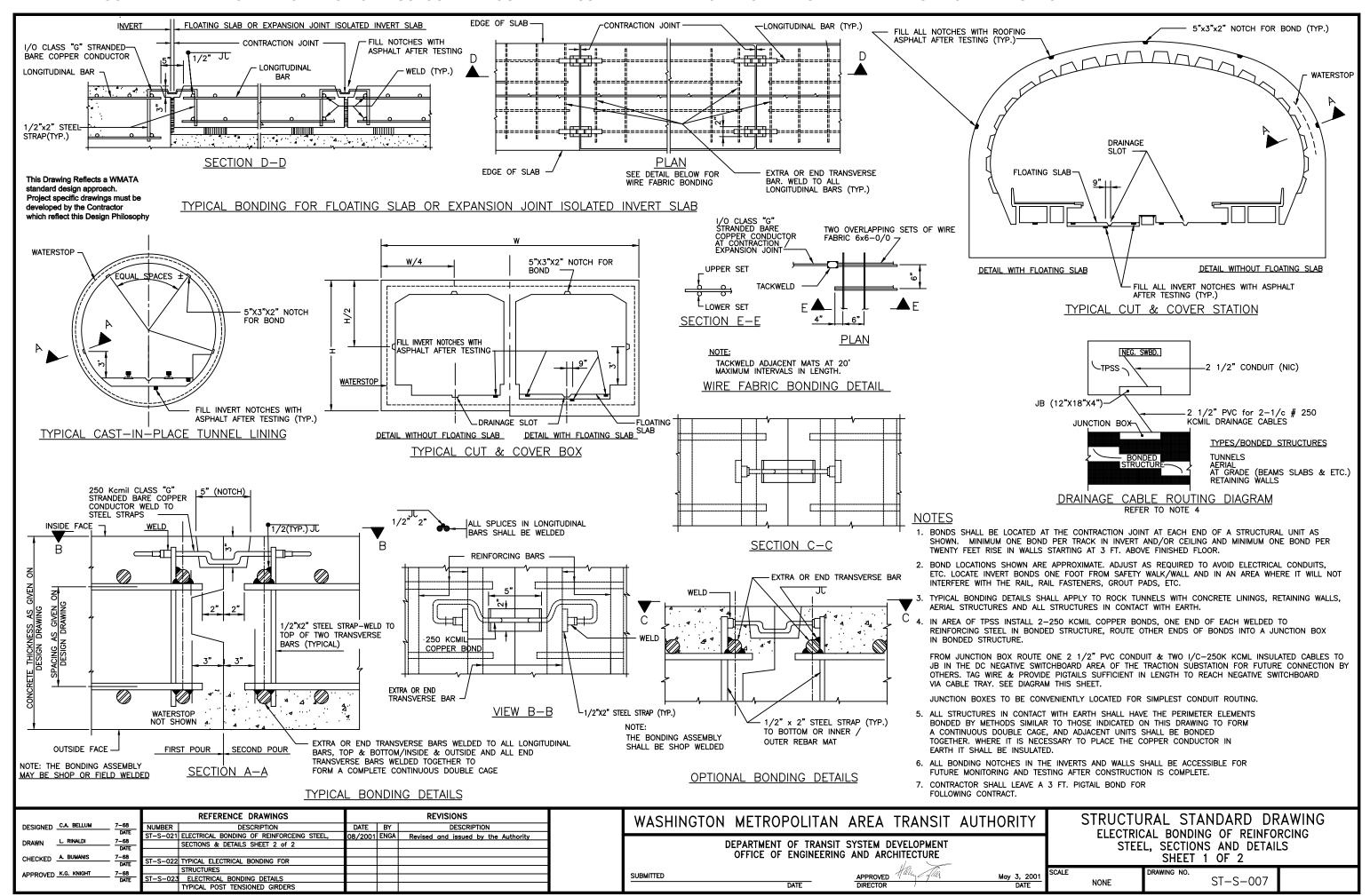
SCALE DRAWING NO. S-29







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LOADS LIVE LOADS (LL) AND OTHER LOADS DESIGN LOADING										
	STRUCTURES	DESIGN LOADING COMBINATIONS AND ALLOWABLE								
	STRUCTURES DEAD LOADS		VERTICAL	HORIZONTAL	UNIT STRESSES					
DECK STRUCTURE	DECK ELEMENTS (DECKING AND HORIZONTAL SUPPORTING FRAMEWORK)	OWN WEIGHT	ROADWAY LOADS 1. BASIC LOADING (LL) HS 25-44 APPLICABLE REFERENCES. * ART. 3.7, 3.11, 3.23 TO 3.29 2. IMPACT (I) ART. 3.8 3. NUMBER AND WIDTH TRAFFIC LANES DRAWINGS OR SPECIFIED. 4. ART. 3.12 (REDUCTION IN LOAD DOES NOT APPLY.) OPERATING LOADS FROM CONSTRUCTION EQUIPMENT (LL) WITH NOT LESS THAN 50% IMPACT. SIDEWALK AND PEDESTRIAN ISLAND LOADS (LL) 250 PSF, OR VEHICULAR LOADS, WHICHEVER ARE GREATER.	LONGITUDINAL FORCES (LF) ART. 3.9 EXCEPT REFERENCE TO ART. 3.12 WIND LOADS (W) 20 PSF ON EXPOSED AREA OF VEHICLES AND EQUIPMENT. BUT NOT LESS THAN 100 LBS. PER LINEAR FOOT OF DECK STRUCTURE APPLIED NORMAL TO THE DIRECTION IN WHICH LENGTH IS MEASURED. LATERAL EARTH (E) AND HYDROSTATIC (H) PRESSURE, SAME AS FOR EXCAVATION RETAINING STRUCTURES.	LOADING SHALL CONSIST OF THE FOLLOWING: DL + LL + I + E + H AT 100% OF UNIT STRESS -OR- DL + LL + I + E + H + LF + W AT 125% OF UNIT STRESS, WHICHEVER IS GREATER. NOTE: THE VALUE OF LL IS THE MAXIMUM TOTAL LIVE LOAD OBTAINED BY COMBINING THE VARIOUS LIVE LOADS THAT MIGHT EXIST AT ONE TIME.					
		UTILITY FACILITIES TO								
	RAILINGS	OWN WEIGHT	(LL) ART. 3.14							
	CURBS AND SIDEWALKS	OWN WEIGHT	150 PSF	(LL) ART. 3.14.1 AND 3.14.2						
TAINING STRUCTURE	WALL SYSTEM (ELEMENTS 'IN CONTACT WITH EARTH, EXCEPT LAGGING)	REACTIONS FROM ALL LIVE LOADS, EXCLUDING IMPACT ON DECK STRUCTURE (LL) OWN WEIGHT AND REACTIONS FROM DEAD LOADS OF		LATERAL EARTH PRESSURE DUE TO WEIGHT OF SOIL AND SURCHARGE (E) HYDROSTATIC PRESSURE (H) AXIAL LOADS FROM END BULKHEAD WHERE APPLICABLE (E) AND (H)	DL + LL + E + H AT 120% OF UNIT STRESS					
EXCAVATION - RETAI	MAIN MEMBERS (MEMBERS CARRYING DIRECT LOADS INCLUDING STRUTS AND WALES)	DECK STRUCTURE AND BRACING SYSTEM		SIMPLE BEAM REACTIONS FROM WALL SYSTEMS (E) AND (H) AXIAL LOADS FROM END WALLS WHERE APPLICABLE (E) AND (H)	DL + LL + E + H AT 100% OF UNIT STRESS					
	SECONDARY BRACING		AXIAL LOAD EQUAL TO 2% OF THE DESIGN	AXIAL LOAD IN THE BRACED MAIN MEMBER	120% OF UNIT STRESS					

^{*} REFERENCES ARE TO ARTICLES IN 'THE STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES', SIXTEENTH EDITION OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICALS 1996.

CRITERIA

LUMBER:

- TEMPORARY EARTH RETAINING AND DECK STRUCTURES SHALL BE DESIGNED
 IN ACCORDANCE WITH THE REQUIREMENTS SHOWN ON THIS DRAWING, ON THE
 DRAWING TITLED 'LATERAL PRESSURES FOR THE DESIGN OF TEMPORARY EARTH
 RETAINING STRUCTURES', AND APPLICABLE SPECIFICATIONS.
- 2. UNLESS MODIFIED BY THE CONTRACT DRAWINGS AND SPECIFICATIONS THE STRUCTURAL DESIGN SHALL BE GOVERNED BY THE CURRENT EDITIONS OF THE FOLLOWING MANUALS, CODES OR SPECIFICATIONS.

ROADWAY DECK: 'STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS' EXCEPT DEFLECTION DUE TO LIVE LOAD PLUS IMPACT SHALL NOT EXCEED 1/600

OF THE SPAN

TEMPORARY RETAINING STRUCTURES AND OTHER TEMPORARY STRUCTURES:

STEEL: 'SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS'
OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION

WELDING: STRUCTURAL WELDING CODE OF THE AMERICAN

WELDING SOCIETY' D1.1.

REINFORCING CONCRETE: BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE OF THE AMERICAN CONCRETE INSTITUTE

'NATIONAL DESIGN SPECIFICATION FOR STRESS-GRADE LUMBER AND ITS FASTENERS' OF THE NATIONAL FOREST

PRODUCTS ASSOCIATION

- 3. THE CONTRACTOR SHALL SUBMIT FOR REVIEW BY THE ENGINEER COMPLETE COMPUTATIONS AND WORKING DRAWINGS FOR TEMPORARY STRUCTURES. THE DESIGN SHALL BE IN ACCORDANCE WITH THE GIVEN LOADS ON THIS SHEET AND GOOD ENGINEERING PRACTICE, AND WILL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 4. EARTH RETAINING STRUCTURES SHALL BE ANALYZED FOR THE VARIOUS CONDITIONS THAT MAY OCCUR DURING THE LIFE OF THE STRUCTURE. SUCH AS THE SEVERAL STAGES OF EXCAVATION, CONSTRUCTION, INSTALLATION, REMOVAL AND RELOCATION OF STRUTS. THE WORKING DRAWINGS SHALL SHOW CONSTRUCTION SEQUENCE AND DETAILS OF POSTING, DIAGONAL LACING, WEB STIFFENERS, ETC.
- 5. WHERE THE LOADING CONDITIONS ON OPPOSITE SIDES OF AN EXCAVATION ARE NOT EQUAL, THE STABILITY OF THE TEMPORARY RETAINING STRUCTURE SHALL BE ANALYZED TO TAKE THIS CONDITION INTO ACCOUNT.
- 6. SOLDIER BEAMS MAY BE CONSIDERED FULLY BRACED IN THE PLANE OF THE WALL.
- 7. THE LOADS IN WALES AND STRUTS FOR FLEXIBLE OR RIGID WALL SYSTEMS SHALL BE COMPUTED BY ASSUMING THE WALL TO BE HINGED AT A SUPPORT POINT BELOW THE BOTTOM OF THE EXCAVATION AND AT EACH STRUT EXCEPT THE TOP ONE.
- 8. STRUTS SHALL BE PRESTRESSED TO 50% OF THEIR MAXIMUM DESIGN LOAD.
- 9. ALL COMPRESSION MEMBER CONNECTIONS:
 - SHALL BE DESIGNED FOR THE MAXIMUM COMPRESSIVE LOAD (CLD), COMBINED WITH GREATER OF THE ACTUAL SHEAR OR SHEAR EQUAL TO 10% CLD.
 - b) THE CONNECTIONS SHOULD BE DESIGNED FOR THE GREATER OF ACTUAL TENSION OR TENSION EQUAL TO 10% CLD AND COMBINED WITH THE GREATER OF ACTUAL SHEAR OR SHEAR EQUAL TO 10% CLD.
- 10. WHERE THE BOTTOM OF THE TRACK-DRAIN TRENCH IS BELOW A 1-VERTICAL, TO 2-HORIZONTAL INFLUENCE LINE FROM THE BOTTOM OF THE INVERT AT THE SIDE OF EXCAVATION. ADEQUATE BRACING TO RESIST LATERAL PRESSURES SHALL BE INSTALLED IN THE TRACK-DRAIN TRENCH.
- 11. THE CONTRACTOR MAY SUBMIT ALTERNATIVE TEMPORARY EARTH-SUPPORT STRUCTURES FOR REVIEW BY THE ENGINEER.

			REFERENCE DRAWINGS			REVISIONS
DESIGNED A. BUMANIS	9-68 DATE	NUMBER	DESCRIPTION	DATE	BY	DESCRIPTION
				08/2001	ENGA	Revised and issued by the Authority
DRAWN R.L.	12-68 DATE					
CHECKED A.B.	1-69					
CHECKED A.B.	DATE					
APPROVED KNIGHT	1-69					
AFFROVED	DATE					
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WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

DEPARTMENT OF TRANSIT SYSTEM DEVELOPMENT OFFICE OF ENGINEERING AND ARCHITECTURE

DATE

SUBMITTED

PROVED HALL THE

STRUCTURAL STANDARD DRAWING CRITERIA FOR THE DESIGN OF TEMPORARY STRUCTURES

ST-S-009

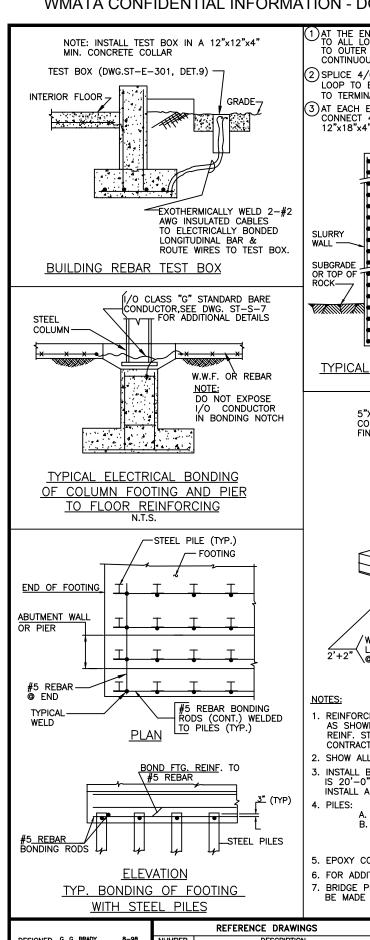
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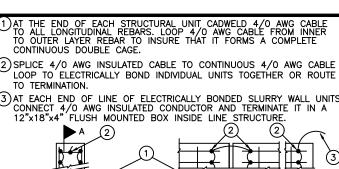
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CADWFI D

TRANSVERSE

/O AWG CARLE

BAR (TYP.)

LOOPED FROM

INNER TO OUTER

SLURRY WALL ELECTRICAL BONDING

REBAR LAYERS

-ROTTOM

OF WALL

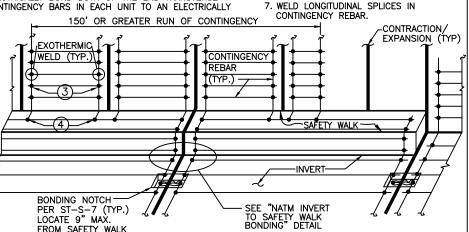
1. ELECTRICAL BONDING OF CONTINGENCY REBAR IS REQUIRED ONLY FOR A CONTINUOUS RUN OF 150 FEET OR MORE

2. ONLY ELECTRICALLY BONDED REBAR IS SHOWN FOR CLARITY. 3. ELECTRICALLY BOND BOTH OF THE END TRANSVERSE BARS

IN EACH UNIT TO EACH LONGITUDINAL BAR AS SHOWN. 4. ELECTRICALLY BOND BOTH OF THE END TRANSVERSE CONTINGENCY BARS IN EACH UNIT TO AN ELECTRICALLY

CONTINUOUS LONGITUDINAL BAR IN THE SAFETY WALK/INVERT AS SHOWN. 5. A CONTINUOUS RUN START/ENDS WITH A FOUR FOOT (4') OR GREATER BREAK IN CONTINGENCY REBAR.

6. ALL INVERT REBAR SHALL BE ELECTRICALLY BONDED PER ST-S-7.



ELECTRICAL BONDING REQUIREMENTS FOR "NATM" TUNNEL REBAR (INVERT & CONTINGENCY REBAR)

EXPANSION OR CONTRACTION JOINT. 5"X3"X2" NOTCH FOR SEE SECTION A-A, DWG. ST-S-7. COPPER BOND 1" ABOVE FINISH GRADE ADD AN EXTRA VERT. BAR IN EACH FACE © END OF WALL & WELD TO ALL LONGITUDINAL BARS. _END OF WALL FLUSH MOUNTED STRAY CURRENT TEST BOX AT EACH END OF WALL 1/2"x2"x LENGTH AS REQ'D ≶STEEL STRAP. WELD TO TOP & BOTTOM AND I.S. & O.S. LAYERS OF END TRANSVERSE BARS #2 AWG INSULATED WIRE, THERMIT WELDED TO EACH LAYER OF REINF. & PROVIDE 2'-0" PIGTAIL IN TEST BOX. WELD (TYP) #4 TRANSVERSE BAR END OF WALLS (T&B) & WELD TO WELD ALL SPLICES IN LONGITUDINAL BARS (TYP.) ALL LONGITUDINAL BARS MIDPOINT OF LAP. TYPICAL BONDING FOR RETAINING WALL DETAIL

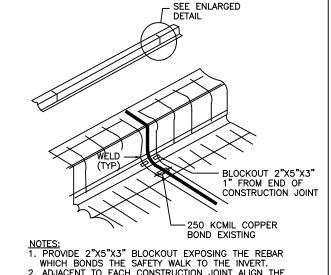
NO SCALE

NOTES:

REINFORCING IN RETAINING WALLS, FOOTINGS, ABUTMENTS & OTHER WALLS IS TO BE BONDED AS SHOWN ON THIS DWG. BOND ALL CONTIGUOUS COMPONENTS EITHER BY DIRECT WELDING OF LAPPED REINF. STEEL AT CONSTRUCTION JOINTS OR AS PER SECTION A-A ON DWG. ST-S-7 AT EXPANSION & CONTRACTION JOINTS.

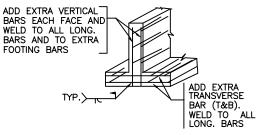
2. SHOW ALL WELDING & REINF. STEEL REQ'D FOR ELECTRICAL BONDING ON SHOP DRAWINGS.

- 3. INSTALL BONDING NOTCHES ON THE TRACK SIDE OF THE RETAINING WALLS AT JOINTS,...WHERE THE WALL IS 20'-0" OR LESS USE ONE BOND AT FINISHED GROUND ELEVATION. FOR WALLS GREATER THAN 20'-0" INSTALL A SECOND BOND HALFWAY BETWEEN THE FOOTING AND THE FINISHED GROUND ELEVATION.
- 4. PILES:
 - A. REINFORCEMENT FROM PRECAST CONCRETE PILES NEED NOT BE BONDED. B. SOLDIER PILES SHALL BE BONDED TOGETHER USING 4/0 AWG INSULATED CONDUCTOR EXOTHERMICALLY WELDED TO PILES. AT EACH END OF PILES TERMINATE THE 4/0 CONDUCTOR IN A BOX INSIDE THE LINE STRUCTURE.
- 5. EPOXY COATED REBAR & WIRE MESH SHALL NOT BE ELECTRICALLY BONDED.
- 6. FOR ADDITIONAL BONDING NOTES SEE DWG. ST-S-7.
- 7. BRIDGE PIERS AND PIER PILES, AND REINFORCEMENT IN THE FOOTING AND COLUMNS SHALL NOT BE MADE ELECTRICALLY CONTINUOUS AND SHALL NOT BE BONDED TO THE SUPER STRUCTURE.

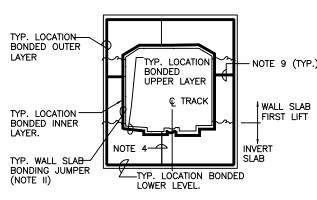


WHICH BONDS THE SAFETY WALK TO THE INVERT.
ADJACENT TO EACH CONSTRUCTION JOINT ALIGN THE END SAFETY WALK TRANSVERSE REBARS WITH THE END INVERT REBARS AND WELD AS SHOWN.

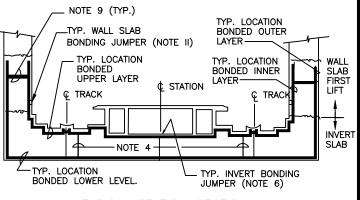
"NATM" INVERT TO SAFETY WALK BONDING



DETAIL AT JOINTS OF RETAINING WALL AND ABUTMENT



TYPICAL TUNNEL SECTION



TYPICAL STATION SECTION

NOTES:

GENERAL

- 1. ALL WELDING FOR ELECTRICAL BONDING OF REINFORCING STEEL SHALL CONFORM TO ANSI/AWS D1,1-92 AND ANSI/AASHTO/AWS D1-S-88 WITH 1992, 1995 AND LATER REVISIONS, USING ETOXX ELECTRODES.
- 2. FIELD ADJUSTMENTS TO BE AS REQUIRED TO AVOID INTERFERENCE.

INVERT SLAB

- 3. ALL LONGITUDINAL BARS IN UPPER AND LOWER LAYERS SHALL BE MADE ELECTRICALLY CONTINUOUS BY WELDING AT LAPS.
- 4. UPPER AND LOWER LAYERS SHALL BE BONDED TOGETHER BOTH SIDES OF EACH CONTRACTION JOINT.
- 5. TRANSVERSE BARS IN UPPER AND LOWER LAYERS BOTH SIDES OF EACH CONTRACTION JOINT SHALL BE MADE ELECTRICALLY CONTINUOUS BY WELDING LAPS. ALL LONGITUDINAL BARS SHALL BE WELDED TO THESE TRANSVERSE BARS.
- 6. PROVIDE BONDING JUMPER ACROSS EACH CONTRACTION AND EXPANSION JOINTS.

WALL SLAB

- 7. LONGITUDINAL BARS IN INNER AND OUTER LAYERS SHALL BE MADE ELECTRICALLY CONTINUOUS BY WELDING AT LAPS.
- 8. AT CONTRACTION OR EXPANSION JOINTS ALL LONGITUDINAL BARS SHALL BE WELDED TO THE END TRANSVERSE BAR. THE END TRANSVERSE BARS OF EACH MAT SHALL BE WELDED AT THE LAPS TO MAKE A CONTINUOUS ELECTRICAL LOOP. THE END TRANSVERSE BARS SHALL BE CONNECTED TO EACH OTHER BY JUMPER BARS WFIDED TO FACH LAYER.
- 9. INNER AND OUTER LAYERS SHALL BE BONDED TOGETHER BOTH SIDES OF EACH CONTRACTION JOINT.
- IO VERTICAL BARS IN INNER AND OUTER LAYERS BOTH SIDES OF CONTRACTION JOINT SHALL BE MADE ELECTRICALLY CONTINUOUS BY WELDINGS AT LAPS
- I.1. PROVIDE BONDING JUMPER ACROSS EACH CONTRACTION JOINT AT A HEIGHT 1 FOOT ABOVE FINISH FLOOR OR FINISH GRADE.

ELECTRICALLY BONDED CONTINUOUS DOUBLE CAGE **EXAMPLES & GUIDES**

REVISIONS DESCRIPTION DESCRIPTION ELECTRICAL BONDING OF REINFORCING
STEEL, SECTIONS & DETAILS 08/2001 ENGA Revised and issued by the Authority SHEET 1 OF 2. CHECKED J. BUMANI -S-022 TYPICAL ELECTRICAL BONDING FOR STRUCTURES APPROVED R. FENG 23 ELECTRICAL BONDING DETAILS

TYPICAL POST TENSIONED GIRDERS

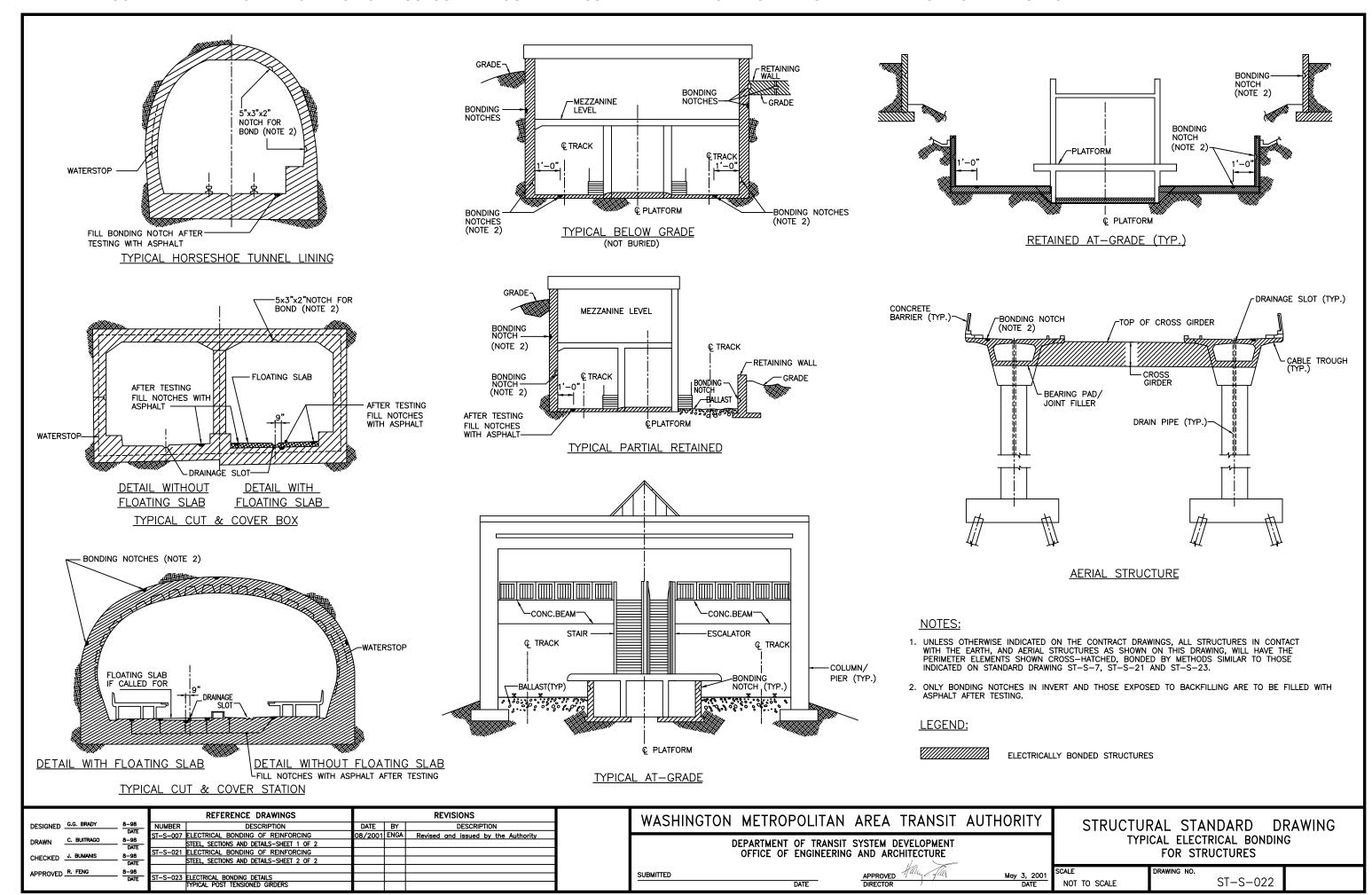
WASHINGTON METROPOLITAN AREA TRANSIT AUTHORITY

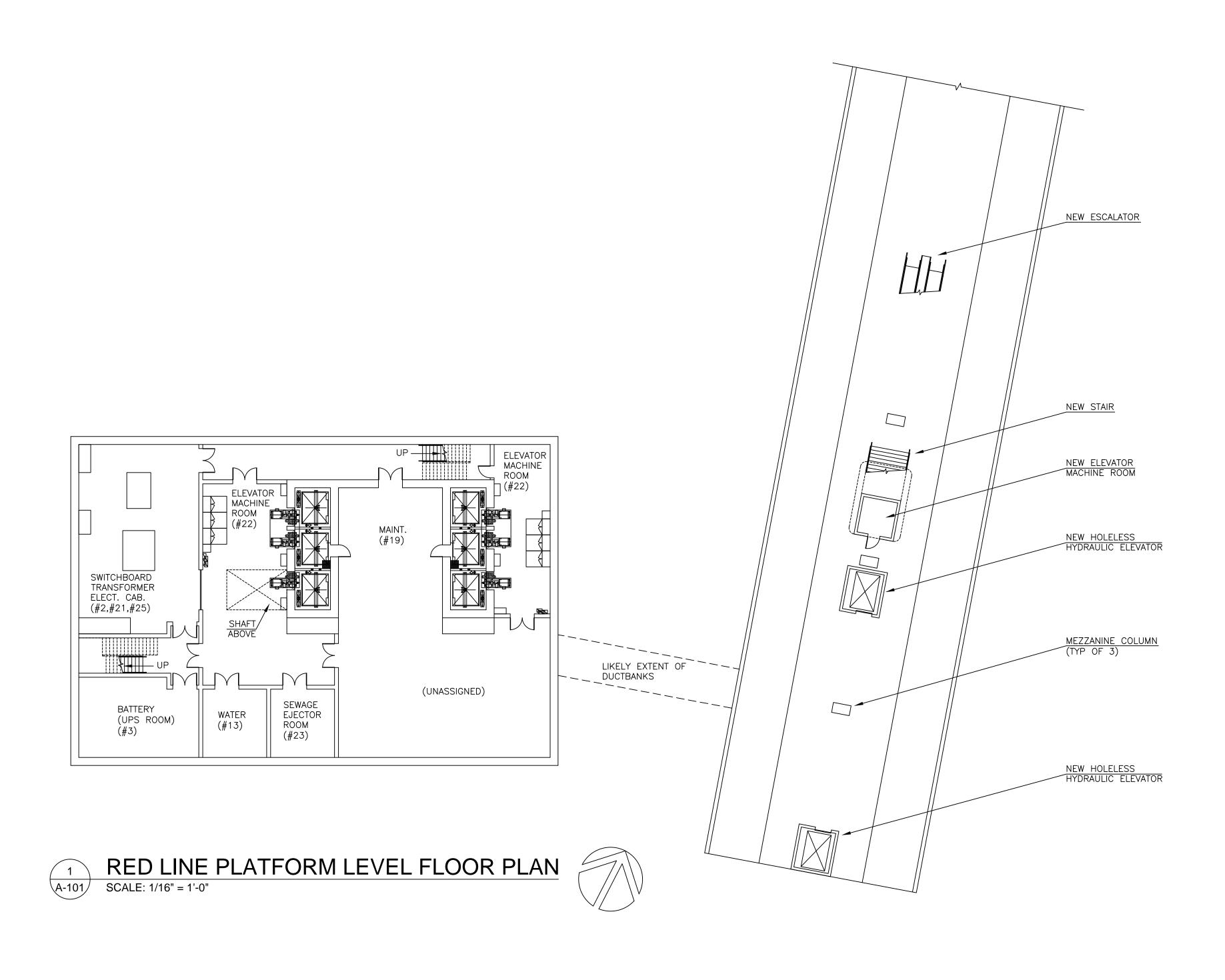
DEPARTMENT OF TRANSIT SYSTEM DEVELOPMENT OFFICE OF ENGINEERING AND ARCHITECTURE

SUBMITTED DATE May 3, 200 DATE

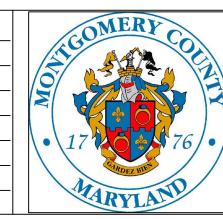
STRUCTURAL STANDARD DRAWING ELECTRICAL BONDING OF REINFORCING STEEL SECTIONS & DETAILS, SHEET 2 OF 2

AS NOTED ST-S-021





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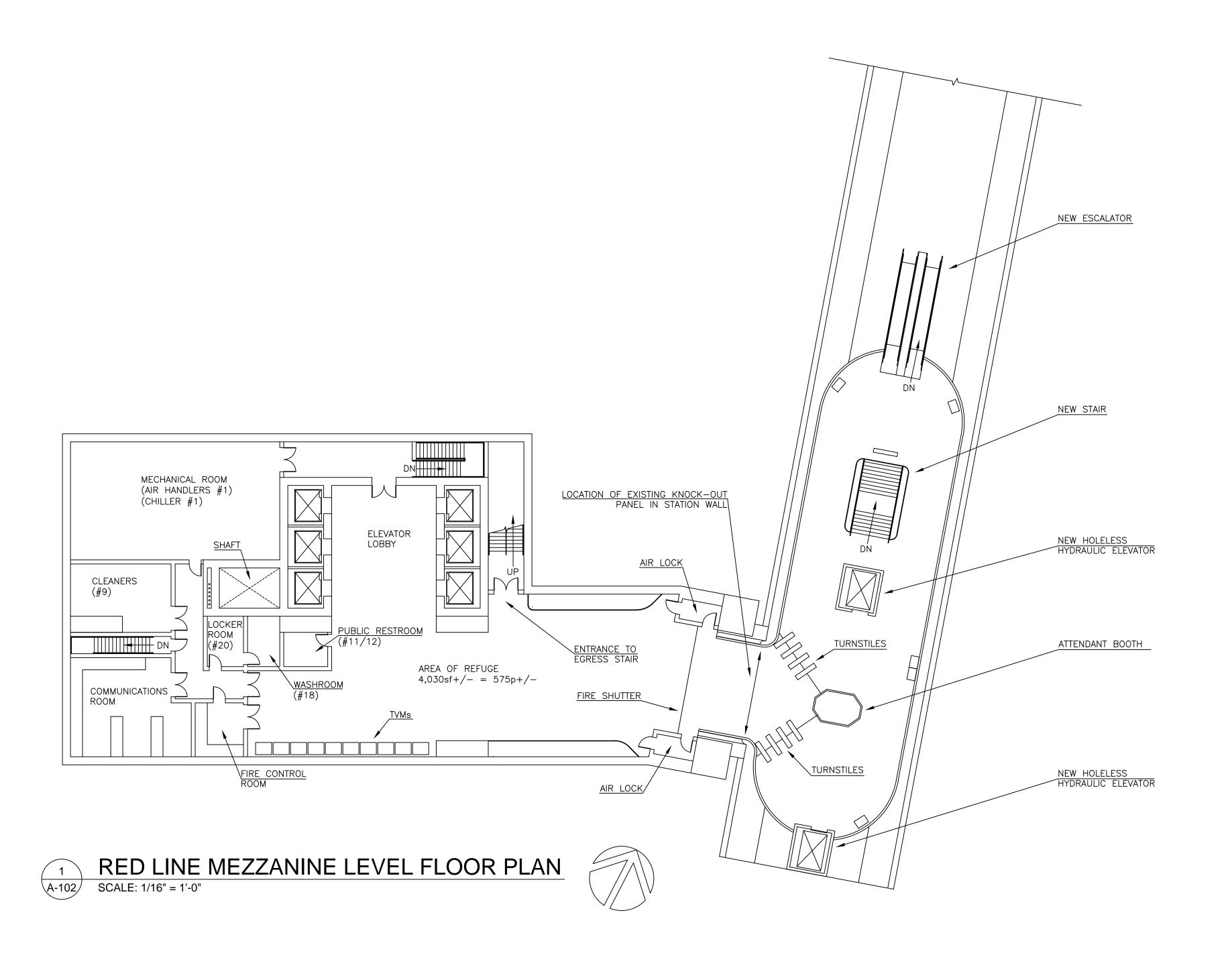
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OFFICE OF ENGINEERING SERVICE

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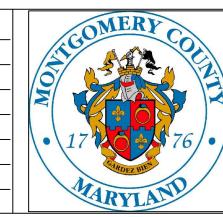
Rummel, Klepper & Kahl, LLP
81 MOSHER STREET | BALTIMORE, MD 21217
PH: (410) 728-2900 | FAX: (410) 728-3160 | WHITMAN, REQUARDT
& ASSOCIATES, LLP
and South Caroline Street, Baltimore, Maryland 21321

BETHESDA	STATION -	- SOUTH	ENTRANCE						
RED LINE	RED LINE PLATFORM LEVEL FLOOR PLAN								

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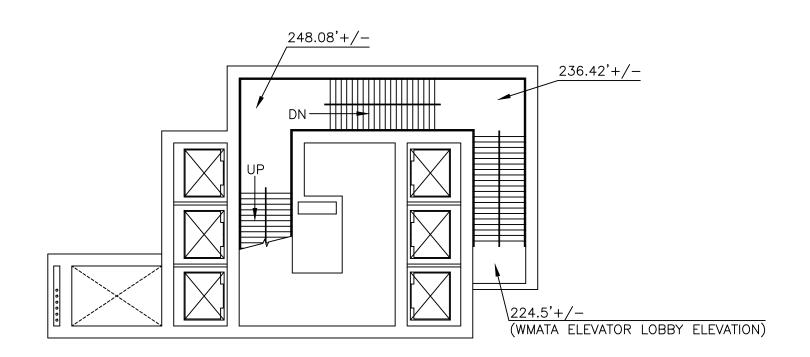
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RED LINE MEZZANINE LEVEL FLOOR PLAN

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EMERGENCY STAIR LANDING 1 - FLOOR PLAN

SCALE: 1/16" = 1'-0"



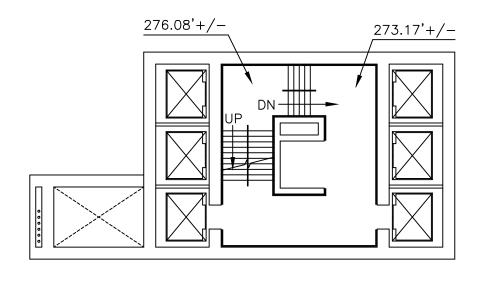
267.33'+/-

255.67'+/-/

EMERGENCY STAIR LANDING 2 - FLOOR PLAN SCALE: 1/16" = 1'-0"

264.42'+/-



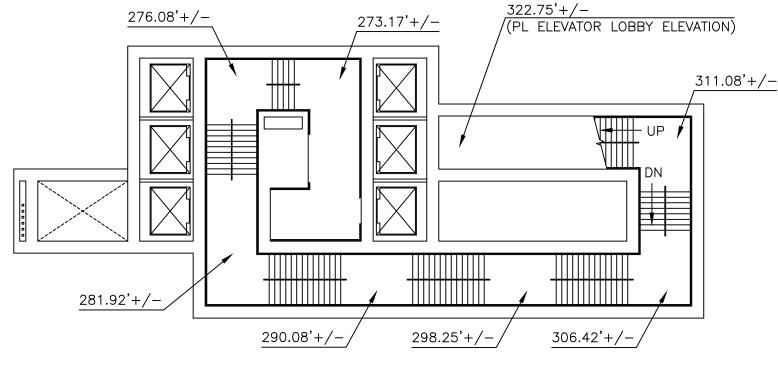




EMERGENCY STAIR LANDING 3 - FLOOR PLAN

SCALE: 1/16" = 1'-0"





EMERGENCY STAIR LANDING 4 - FLOOR PLAN SCALE: 1/16" = 1'-0" A-103



CONTRACT NO. XXXXXX

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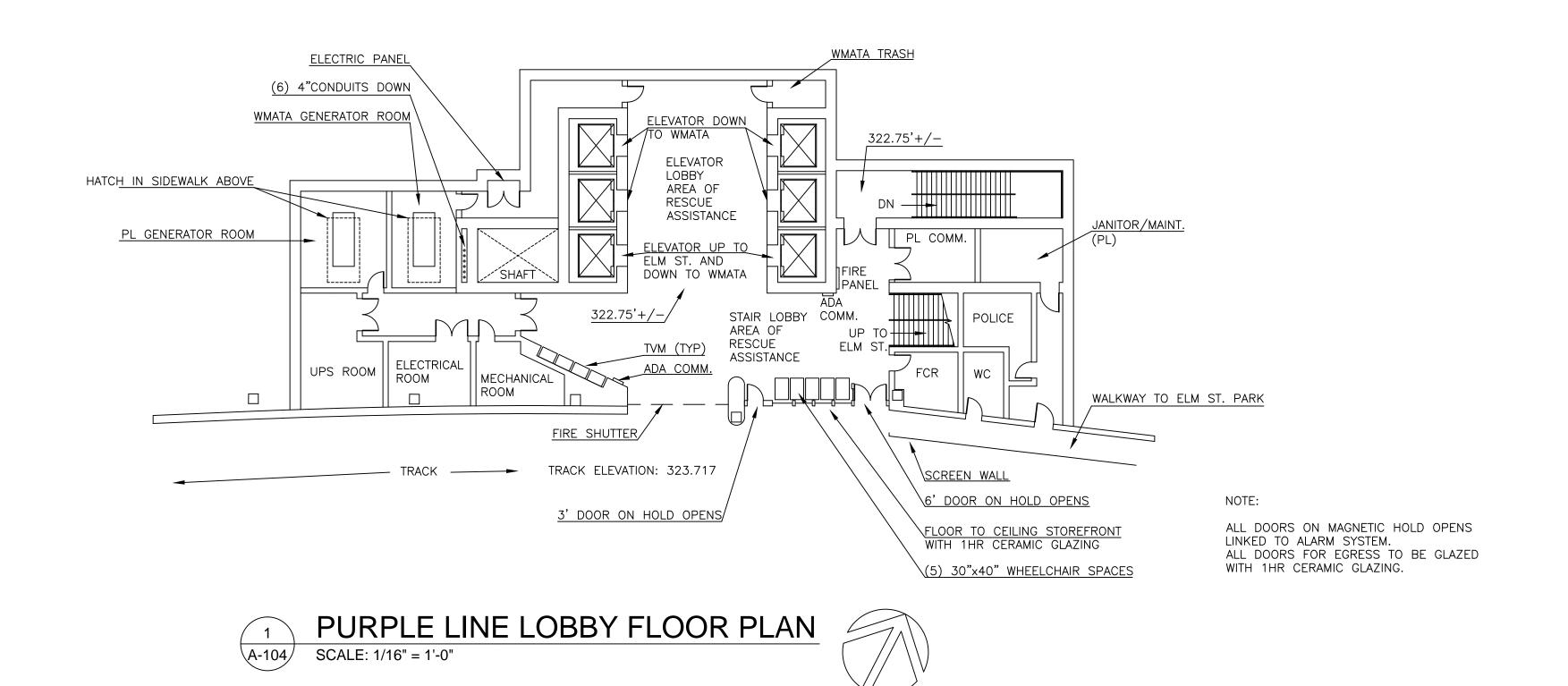
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BETHESDA STATION - SOUTH ENTRANCE

EMERGENCY STAIR INTERMEDIATE LANDING FLOOR PLANS

DRAWING NO. SCALE A - 103AS NOTED



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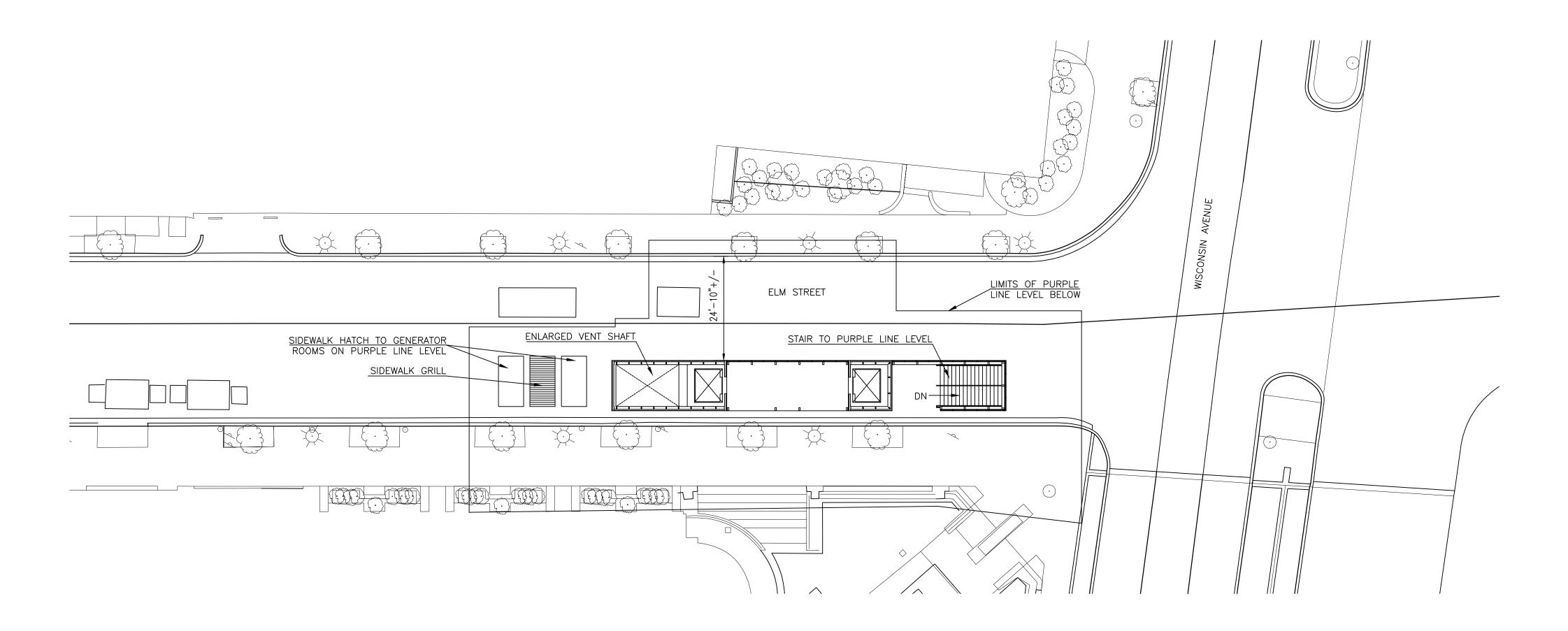
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301 South Caroline Street, Baltimore, Maryland \$1391

BETHESDA STATION — SOUTH ENTRANCE

PURPLE LINE LORBY ELOOP PLAN

PURPLE LINE LOBBY F	LOOR PLAN
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ELM STREET FLOOR PLAN A-105 SCALE: 1/16" = 1'-0"

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REVISIONS DESIGNED DESCRIPTION DATE BY CHECKED APPROVED _





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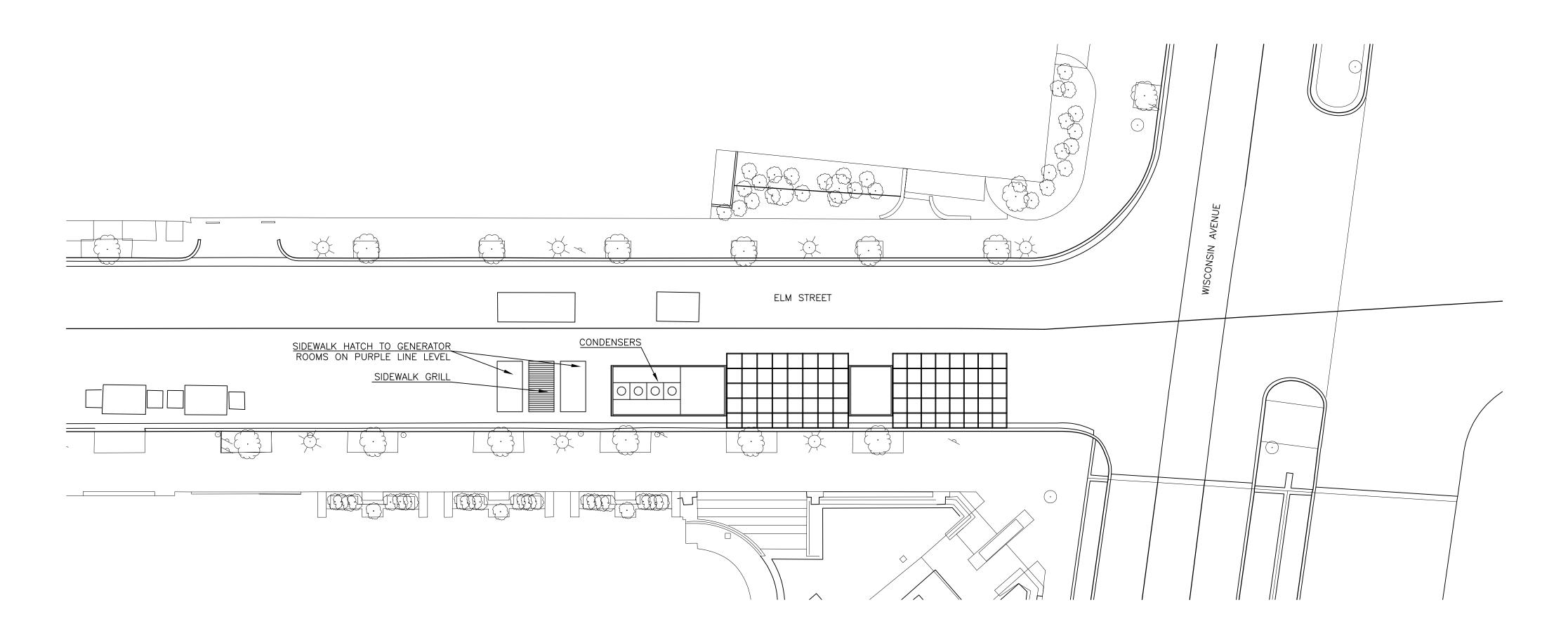
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& ASSOCIATES, LLP
Sont South Caroline Street, Baltimore, Maryland 21291

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ELM STREET FLOOR PLAN

SCALE DRAWING NO. A - 105AS NOTED



ELM STREET ROOF PLAN A-106 SCALE: 1/16" = 1'-0"

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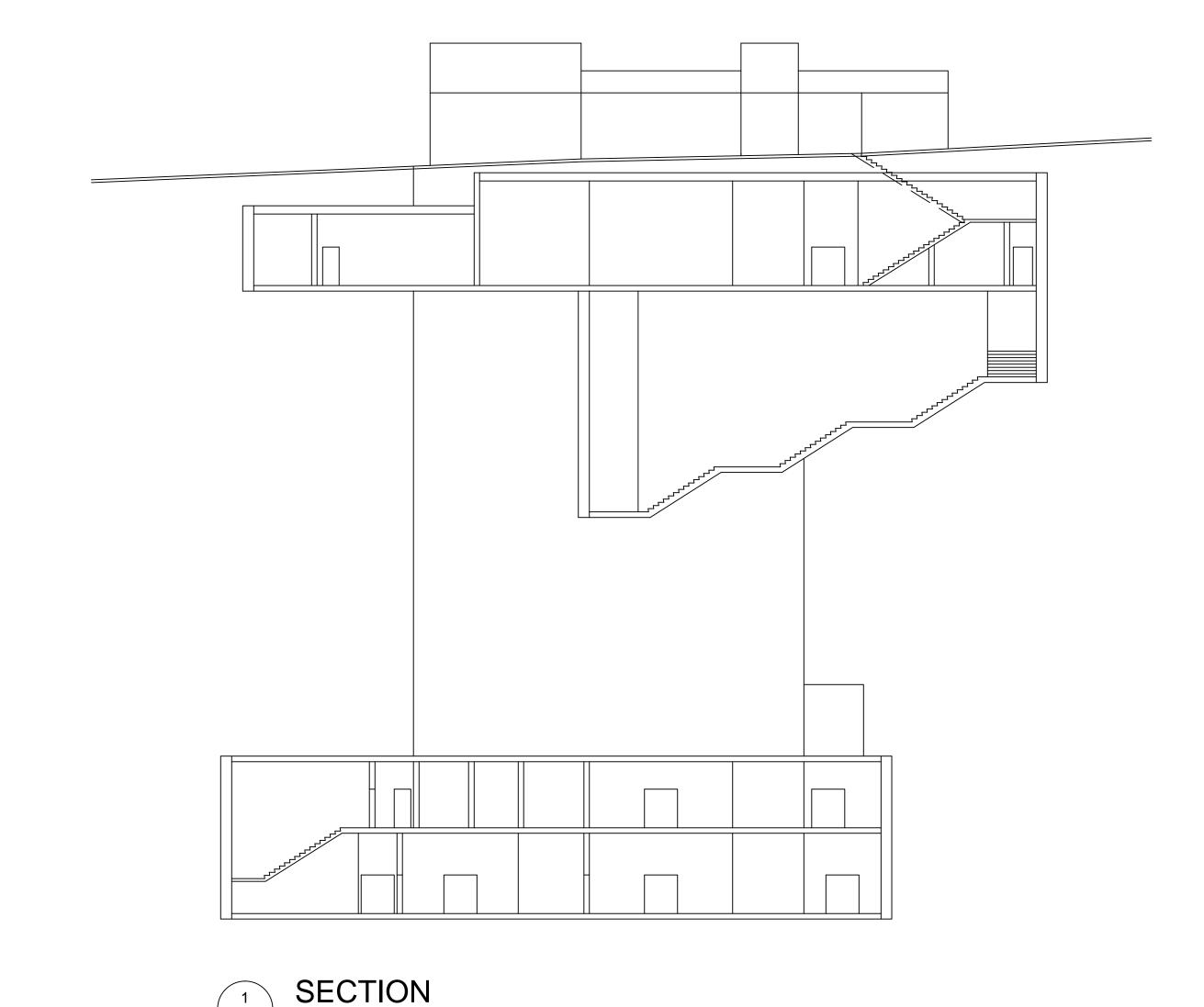
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ELM STREET ROOF PLAN

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A-301 SCALE: 1/16" = 1'-0"





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